



— Technical Requirements

under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area, and/or manure storage facility(ies)

CB USE ONLY	Application number RA21043	Legal land description NE 15-42-24 W4M
<input checked="" type="checkbox"/> Approval <input type="checkbox"/> Registration <input type="checkbox"/> Authorization <input type="checkbox"/> Amendment		

APPLICATION DISCLOSURE

This information is collected under the authority of the *Agricultural Operation Practices Act (AOPA)*, and is subject to the provisions of the *Freedom of Information and Protection of Privacy Act*. This information is public unless the NRCB grants a written request that certain sections remain private.

Any construction prior to obtaining an NRCB permit is an offence and is subject to enforcement action, including prosecution.

I, the applicant, or applicant's agent, have read and understand the statements above, and I acknowledge that the information provided in this application is true to the best of my knowledge.

Feb 15/22
 Date of signing
 Double T Cattle
 Corporate name (if applicable)

[Signature]
 Signature
 Joe Thalen
 Print name

GENERAL INFORMATION REQUIREMENTS

Proposed facilities: list all proposed confined feeding operation facilities and their dimensions. Indicate whether any of the proposed facilities are additions to existing facilities. (attach additional pages if needed)	
Proposed facilities	Dimensions (m) (length, width, and depth)
6 feedlot pens Total Area 52,256 sq. m with	4-32.97x304.8 & 2-32.97x182.88
Total area under roof 15,057 sq. m (scraper alley and feed alley)	4-9.5x304.8 & 2-9.5x182.88
Catch basin 'L' shaped <i>Refer to drawing for exact size.</i>	43m x 110m & 52m x 34m <i>Approx. 56.64 x 122.38 38.61 x 52.21</i>

Existing facilities: list ALL existing confined feeding operation facilities and their dimensions		
Existing facilities	Dimensions (m) (length, width, and depth)	NRCB USE ONLY
Sow Barn	27mx40m	
Note: only the approximate east half of the sow barn retains the original concrete floor, though the north pit wall was renovated to include drive through overhead doors. This barn is further discussed on the next page and in Decision Summary RA21043.		
NRCB USE ONLY		

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Part 2 – Technical Requirements

Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area, and/or manure storage facility(ies)

If a new facility is replacing an old facility, please explain what will happen to the old facility and when. N/A

Old pig barn is being used for livestock processing building.

Livestock processing facilities are not considered to be manure collection areas or manure storage facilities; they are ancillary structures that do not require a permit under the AOPA.

Construction completion date for proposed facilities December 31, 2025

Additional information

Livestock numbers: Complete only if livestock numbers are different from what was identified in the Part 1 application. Note: if livestock numbers increase in your Part 2 application, a new Part 1 application must be submitted which may result in a loss of priority for minimum distance separation (MDS).

Livestock category and type (Available in the Schedule 2 of the Part 2 Matters Regulation)	Permitted number	Proposed increase or decrease in number (if applicable)	Total
Beef, Cow/finisher	0	4500	4500
Sow Farrow to Finish	700	700	0
The status of the CFO's permits and its associated livestock capacity are discussed in Decision Summary RA21043.			

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Part 2 – Technical Requirements



Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area, and/or manure storage facility(ies)

DECLARATION AND ACKNOWLEDGMENT OF APPLICANT CONCERNING WATER ACT LICENCE

Issued by Alberta Environment and Parks (AEP) for a confined feeding operation (CFO)

Date and sign one of the following four options

OPTION 1: Applying through the NRCB for both the AOPA permit and the Water Act licence

I **DO** want my water licence application coupled to my AOPA permit application.

Signed this ____ day of _____, 20____.

Signature of Applicant or Agent

OPTION 2: Processing the AOPA permit and Water Act licence separately

1. I (we) acknowledge that the CFO will need a new water licence from AEP under the *Water Act* for the development or activity proposed in this AOPA application.
2. I (we) request that the NRCB process the AOPA application **independently of** AEP's processing of the CFO's application for a water licence.
3. In making this request, I (we) recognize that, if this AOPA application is granted by the NRCB, the NRCB's decision will not be considered by AEP as improving or enhancing the CFO's eligibility for a water licence under the *Water Act*.
4. I (we) acknowledge that any construction or actions to populate the CFO with livestock pursuant to an AOPA permit in the absence of a *Water Act* licence will **not** be relevant to AEP's consideration of whether to grant the *Water Act* licence application.
5. I (we) acknowledge that any such construction or livestock populating will be at the CFO's sole risk if the *Water Act* licence application is denied or if the operation of the CFO is otherwise deemed to be in violation of the *Water Act*. This risk includes being required to depopulate the CFO and/or to cease further construction, or to remove "works" or "undertakings" (as defined in the *Water Act*).
6. **AS RELEVANT:** I (we) acknowledge that the CFO is located in the South Saskatchewan River Basin and that, pursuant to the *Bow, Oldman and South Saskatchewan River Basin Water Allocation Order* [Alta. Reg. 171/2007], this basin is currently closed to new surface water allocations.

Signed this 10 day of FEB, 2022.

Signature of Applicant or Agent

OPTION 3: Additional water licence not required

1. I (we) declare that the CFO will not need a new licence from AEP under the *Water Act* for the development or activity proposed in this AOPA application.

Signed this ____ day of _____, 20____.

Signature of Applicant or Agent

OPTION 4: Uncertain if Water Act licence is needed; acknowledgement of risk (for existing CFOs only)

1. At this time, I (we) do not know whether a new water licence is needed from AEP under the *Water Act* for the development or activity proposed in this AOPA application.
2. If a new *Water Act* licence is needed, I (we) request that the NRCB process the AOPA application **independently of** AEP's processing of the CFO's application for a water licence.
3. In making this request, I (we) recognize that, if this AOPA application is granted by the NRCB, the NRCB's decision will not be considered by AEP as improving or enhancing the CFO's eligibility for a water licence under the *Water Act*.
4. I (we) acknowledge that any construction or actions to populate the CFO with additional livestock pursuant to an AOPA permit in the absence of a *Water Act* licence will **not** be relevant to AEP's consideration of whether to grant my *Water Act* licence application, if a new water licence is needed.
5. I (we) acknowledge that any such construction or livestock increase will be at the CFO's sole risk if the *Water Act* licence application is denied or if the operation of the CFO is otherwise deemed to be in violation of the *Water Act*. This risk includes being required to depopulate the CFO and/or to cease further construction, or to remove "works" or "undertakings" (as defined in the *Water Act*).
6. **AS RELEVANT:** I (we) acknowledge that the CFO is located in the South Saskatchewan River Basin and that, pursuant to the *Bow, Oldman and South Saskatchewan River Basin Water Allocation Order* [Alta. Reg. 171/2007], this basin is currently closed to new surface water allocations.

Signed this ____ day of _____, 20____.

Signature of Applicant or Agent

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NRCB USE ONLY

ALL SIGNATURES IN FILE YES NO

DATES OF APPROVAL OFFICER SITE VISITS

December 17, 2021, March 24 and 30, 2022	

CORRESPONDENCE WITH MUNICIPALITIES AND REFERRAL AGENCIES

Date deeming letters sent: March 2, 2022

Municipality: _____

letter sent response received written/email verbal no comments received

Alberta Health Services:

letter sent response received written/email verbal no comments received

Alberta Environment and Parks: N/A

letter sent response received written/email verbal no comments received

Alberta Transportation: N/A

letter sent response received written/email verbal no comments received

Alberta Regulatory Services: N/A

letter sent response received written/email verbal no comments received

Other: _____ N/A

letter sent response received written/email verbal no comments received

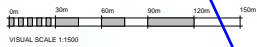
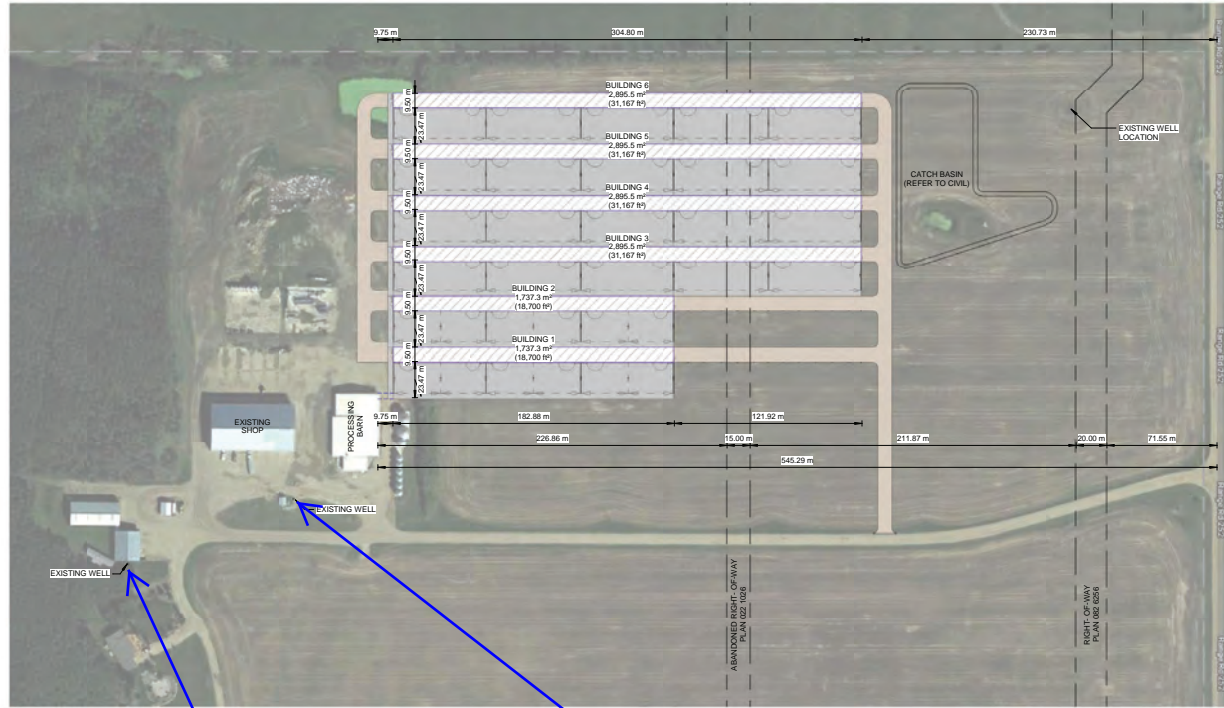
Other: _____ N/A

letter sent response received written/email verbal no comments received

Note: Copies of the responses to the application, other than those noted above, were shared with AHS, AEP, Alberta Transportation, Alberta Regulatory Services and the county. See Decision Summary RA21043.



<https://bit.ly/3wT0B5C>



AEP water well ID # 288714

AEP water well ID # 286659

1 Site Plan
A1.1 1:1500



- NOTES CONCERNING BUILDING LOCATION**
- THIS SITE PLAN IS BASED ON INFORMATION PROVIDED BY THE OWNER, AND NOT A SURVEY OR ACTUAL SITE MEASUREMENTS. ENGINEERS IS TO BE ADVISED BEFORE START OF CONSTRUCTION OF ANY UNKNOWN FEATURES ON THIS OR THE ADJACENT SITES THAT MIGHT IMPACT ON THE PROJECT EITHER DURING CONSTRUCTION OR FUTURE USE.
 - THE "NORTH" ORIENTATION REFERS TO NOMINAL NORTH RATHER THAN TRUE OR MAGNETIC NORTH.
 - ANY DIMENSIONS THAT SHOW THE LOCATION OF EXISTING FEATURES ARE APPROXIMATE ONLY, AND ARE TO BE CONFIRMED BEFORE CONSTRUCTION START AS REQUIRED BY A CERTIFIED ALBERTA LAND SURVEYOR.
 - LANDSCAPING IS SUBJECT TO CHANGE.

SITE PLAN LEGEND

PROPERTY LINE	---
SETBACK LINE	----
RIGHT OF WAY LINE	=====
RIGHT OF WAY HATCH	XXXXXX
PROPOSED BUILDING	▨
FENCE	=====
MAN DOOR	▼
OVERHEAD DOOR	▽
BARRIER FREE ENTRANCE	▽

SITE SCHEDULE

ON SITE	85,197.90 ft ²	7,915.1 m ²
GRAVEL	85,197.90 ft ²	7,915.1 m ²
Grand total	85,197.90 ft ²	7,915.1 m ²

SITE MATERIAL LEGEND

APRON	■
SIDEWALK	■
LIGHT DUTY ASPHALT	■
HEAVY DUTY ASPHALT	■
GRAVEL	■
LANDSCAPING	■
HARD LANDSCAPING	■
MULCH	■

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NO	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
B	ISSUED FOR PRELIMINARY REVIEW	2021-07-22
C	ISSUED FOR PRELIMINARY REVIEW	2021-08-20
D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
E	ISSUED FOR PRELIMINARY REVIEW	2021-08-25
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
G	ISSUED FOR ENGINEERING REVIEW	2021-09-21
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22
I	REVISED CATCH BASIN	2021-10-12
J	REVISED CATCH BASIN	2022-01-31
K	REVISED CATCH BASIN	2022-02-09

Double T Cattle Feedlot

Ponoka County, Alberta

15-15-42-25 W4



Stamp:
PRELIMINARY
NOT FOR CONSTRUCTION

aggregate design studio ltd.
(403) 885-5525 P.O. BOX 1690 BLACKFALDS, AB, T0M 0J0

Site Plan

A1.1

Project number:	A20585
Date:	2022-02-09 12:47:13 PM
Drawn by:	JC
Checked by:	JC
Scale:	As indicated



Water Well Drilling Report

View in Metric Export to Excel

The driller supplies the data contained in this report. The Province disclaims responsibility for its accuracy. The information on this report will be retained in a public database.

GIC Well ID 288714
GoA Well Tag No.
Drilling Company Well ID
Date Report Received 1997/11/06

GOWN ID

Well Identification and Location										Measurement in Imperial
Owner Name	Address			Town	Province	Country				Postal Code
VAN DEPT, WIM	RR3, PONOKA									T4L 1R3
Location	1/4 or LSD	SEC	TWP	RGE	W of MER	Lot	Block	Plan	Additional Description	
	NE	15	42	25	4					
Measured from Boundary of					GPS Coordinates in Decimal Degrees (NAD 83)					
_____ ft from _____					Latitude	52.620020	Longitude	-113.518071	Elevation	_____ ft
_____ ft from _____					How Location Obtained			How Elevation Obtained		
					Not Verified			Not Obtained		

Drilling Information		Type of Work
Method of Drilling Rotary		New Well
Proposed Well Use Stock		

Formation Log		Measurement in Imperial
Depth from ground level (ft)	Water Bearing	Lithology Description
20.00		Brown Shale
115.00		Gray Shale
120.00		Sand & Coal
150.00		Shale
155.00		Gray Sandstone
170.00		Gray Shale
172.00		Hard Shale
217.00		Gray Sandstone
255.00		Gray Shale
260.00		Coal
280.00		Gray Shale
290.00		Coal
300.00		Gray Shale

Yield Test Summary			Measurement in Imperial
Recommended Pump Rate	25.00 igpm		
Test Date	Water Removal Rate (igpm)	Static Water Level (ft)	
1997/09/14	25.00	162.00	

Well Completion				Measurement in Imperial
Total Depth Drilled	Finished Well Depth	Start Date	End Date	
300.00 ft		1997/09/10	1997/09/11	
Borehole				
Diameter (in)	From (ft)	To (ft)		
0.00	0.00	300.00		
Surface Casing (if applicable)		Well Casing/Liner		
Plastic		Plastic		
Size OD :	6.00 in	Size OD :	4.50 in	
Wall Thickness :	0.039 in	Wall Thickness :	0.237 in	
Bottom at :	139.00 ft	Top at :	120.00 ft	
		Bottom at :	300.00 ft	
Perforations				
From (ft)	To (ft)	Diameter or Slot Width (in)	Slot Length (in)	Hole or Slot Interval (in)
217.00	240.00	0.125		8.00
280.00	300.00	0.000		0.00
Perforated by Saw				
Annular Seal Driven				
Placed from 0.00 ft to 139.00 ft				
Amount _____				
Other Seals				
Type		At (ft)		

Screen Type				
Size OD : 0.00 in				
From (ft)	To (ft)	Slot Size (in)		
_____	_____	_____		
Attachment _____				
Top Fittings _____		Bottom Fittings _____		
Pack				
Type _____		Grain Size _____		
Amount _____				

Contractor Certification		Certification No
Name of Journeyman responsible for drilling/construction of well	UNKNOWN NA DRILLER	1
Company Name	TALL PINE DRILLING LTD.	Copy of Well report provided to owner Date approval holder signed

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Water Well Drilling Report

[View in Metric](#) [Export to Excel](#)

The driller supplies the data contained in this report. The Province disclaims responsibility for its accuracy. The information on this report will be retained in a public database.

GIC Well ID 288714
GoA Well Tag No.
Drilling Company Well ID
Date Report Received 1997/11/06

GOWN ID

Well Identification and Location										Measurement in Imperial
Owner Name	Address			Town	Province	Country	Postal Code			
VAN DEPT, WIM	RR3, PONOKA						T4L 1R3			
Location	1/4 or LSD	SEC	TWP	RGE	W of MER	Lot	Block	Plan	Additional Description	
	NE	15	42	25	4					
Measured from Boundary of					GPS Coordinates in Decimal Degrees (NAD 83)					
_____ ft from _____					Latitude 52.620020		Longitude -113.518071		Elevation _____ ft	
_____ ft from _____					How Location Obtained		How Elevation Obtained		Not Obtained	
					Not Verified					

Additional Information										Measurement in Imperial
Distance From Top of Casing to Ground Level _____ in					Is Flow Control Installed _____					
Is Artesian Flow _____					Describe _____					
Rate _____ igpm										
Recommended Pump Rate _____ 25.00 igpm					Pump Installed Yes _____		Depth _____ ft			
Recommended Pump Intake Depth (From TOC) _____ 270.00 ft					Type _____		Make _____		H.P. 3	
					Model (Output Rating) _____					
Did you Encounter Saline Water (>4000 ppm TDS) _____					Depth _____ ft		Well Disinfected Upon Completion _____			
Gas _____					Depth _____ ft		Geophysical Log Taken _____			
					Submitted to ESRD _____					
Additional Comments on Well					Sample Collected for Potability _____		Submitted to ESRD _____			
DRILLER REPORTS DISTANCE FROM TOP OF CASING TO GROUND LEVEL: 1'										

Yield Test			Taken From Ground Level Depth to water level		Measurement in Imperial
Test Date	Start Time	Static Water Level			
1997/09/14	12:00 AM	162.00 ft			
Method of Water Removal					
Type	Pump				
Removal Rate	25.00 igpm				
Depth Withdrawn From	270.00 ft				
If water removal period was < 2 hours, explain why					
			Pumping (ft)	Elapsed Time Minutes:Sec	Recovery (ft)
				1:00	188.00
				2:00	174.00
				3:00	170.00
				4:00	168.00
				5:00	167.00
				6:00	166.00
				7:00	165.50
				8:00	165.00
				9:00	164.58
				10:00	164.67
				14:00	163.58
				20:00	162.08
				35:00	162.00
				75:00	162.00

Water Diverted for Drilling		
Water Source	Amount Taken	Diversion Date & Time
	ig	

Contractor Certification	
Name of Journeyman responsible for drilling/construction of well	Certification No
UNKNOWN NA DRILLER	1
Company Name	Copy of Well report provided to owner Date approval holder signed
TALL PINE DRILLING LTD.	

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Water Well Drilling Report

[View in Metric](#) [Export to Excel](#)

GIC Well ID 286659
GoA Well Tag No.
Drilling Company Well ID
Date Report Received 1997/06/23

The driller supplies the data contained in this report. The Province disclaims responsibility for its accuracy. The information on this report will be retained in a public database.

GOWN ID

Well Identification and Location										Measurement in Imperial	
Owner Name VAN DE PAL, WIM		Address P.O. BOX 6 SITE 19 RR3, PONOKA			Town		Province		Country		Postal Code T4L 1R3
Location	1/4 or LSD NE	SEC 15	TWP 42	RGE 25	W of MER 4	Lot	Block	Plan	Additional Description		
Measured from Boundary of					GPS Coordinates in Decimal Degrees (NAD 83)						
ft from _____					Latitude <u>52.620020</u>		Longitude <u>-113.518071</u>		Elevation _____ ft		
ft from _____					How Location Obtained					How Elevation Obtained	
					Not Verified					Not Obtained	

Drilling Information	
Method of Drilling Rotary	Type of Work New Well
Proposed Well Use Domestic	

Formation Log		Measurement in Imperial
Depth from ground level (ft)	Water Bearing	Lithology Description
20.00		Clay
40.00		Gray Sand
50.00		Gray Shale
60.00		Hard Sandstone
110.00		Gray Shale
120.00		Green Shale
130.00		Gray Sandstone
170.00		Green Shale
250.00		Gray Sandstone
270.00		Green Shale

Yield Test Summary			Measurement in Imperial
Recommended Pump Rate	<u>15.00 igpm</u>		
Test Date	Water Removal Rate (igpm)	Static Water Level (ft)	
1997/04/29	15.00	168.80	

Well Completion				Measurement in Imperial
Total Depth Drilled	Finished Well Depth	Start Date	End Date	
270.00 ft		1997/04/29	1997/04/29	
Borehole				
Diameter (in)	From (ft)	To (ft)		
0.00	0.00	270.00		
Surface Casing (if applicable)		Well Casing/Liner		
Plastic		Plastic		
Size OD :	<u>6.00 in</u>	Size OD :	<u>4.50 in</u>	
Wall Thickness :	<u>0.039 in</u>	Wall Thickness :	<u>0.237 in</u>	
Bottom at :	<u>79.00 ft</u>	Top at :	<u>70.00 ft</u>	
		Bottom at :	<u>270.00 ft</u>	
Perforations				
From (ft)	To (ft)	Diameter or Slot Width (in)	Slot Length (in)	Hole or Slot Interval (in)
210.00	270.00	0.128		8.00
Perforated by <u>Saw</u>				
Annular Seal Driven				
Placed from <u>0.00 ft</u> to <u>79.00 ft</u>				
Amount _____				
Other Seals				
Type		At (ft)		

Screen Type				
Size OD : <u>0.00 in</u>				
From (ft)	To (ft)	Slot Size (in)		
_____	_____	_____		
Attachment _____				
Top Fittings _____		Bottom Fittings _____		
Pack				
Type _____		Grain Size _____		
Amount _____				

Contractor Certification		Certification No	
Name of Journeyman responsible for drilling/construction of well UNKNOWN NA DRILLER		1	
Company Name TALL PINE DRILLING LTD.		Copy of Well report provided to owner Date approval holder signed	

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Water Well Drilling Report

View in Metric Export to Excel

GIC Well ID 286659
GoA Well Tag No.
Drilling Company Well ID
Date Report Received 1997/06/23

GOWN ID

The driller supplies the data contained in this report. The Province disclaims responsibility for its accuracy. The information on this report will be retained in a public database.

Well Identification and Location							Measurement in Imperial		
Owner Name	Address		Town	Province	Country	Postal Code			
VAN DE PAL, WIM	P.O. BOX 6 SITE 19 RR3, PONOKA					T4L 1R3			
Location	1/4 or LSD	SEC	TWP	RGE	W of MER	Lot	Block	Plan	Additional Description
	NE	15	42	25	4				
Measured from Boundary of				GPS Coordinates in Decimal Degrees (NAD 83)			Elevation		
_____ ft from				Latitude 52.620020			Longitude -113.518071		
_____ ft from				How Location Obtained			How Elevation Obtained		
				Not Verified			Not Obtained		

Additional Information				Measurement in Imperial		
Distance From Top of Casing to Ground Level _____ in		Is Artesian Flow _____		Is Flow Control Installed _____		
Rate _____ igpm		Describe _____				
Recommended Pump Rate	15.00 igpm	Pump Installed	_____	Depth	_____	ft
Recommended Pump Intake Depth (From TOC)	208.00 ft	Type	_____	Make	_____	H.P. _____
				Model (Output Rating)		_____
Did you Encounter Saline Water (>4000 ppm TDS) _____		Depth _____ ft		Well Disinfected Upon Completion _____		
Gas _____		Depth _____ ft		Geophysical Log Taken _____		
				Submitted to ESRD _____		
Additional Comments on Well		Sample Collected for Potability _____		Submitted to ESRD _____		
DRILLER REPORTS DISTANCE FROM TOP OF CASING TO GROUND LEVEL: 1'6".						

Yield Test			Taken From Ground Level		Measurement in Imperial	
Test Date	Start Time	Static Water Level	Depth to water level			
1997/04/29	12:00 AM	168.80 ft				
Method of Water Removal			Pumping (ft)	Elapsed Time	Recovery (ft)	
Type Air				Minutes:Sec		
Removal Rate	15.00 igpm			1:00	189.00	
Depth Withdrawn From	208.00 ft			2:00	177.00	
				3:00	174.50	
				4:00	173.50	
				5:00	173.25	
				7:00	172.58	
				8:00	172.33	
				10:00	171.83	
				14:00	171.42	
				20:00	170.00	
				30:00	169.75	
				40:00	169.42	
				60:00	169.17	
				90:00	168.92	
				105:00	168.83	
				120:00	168.75	
If water removal period was < 2 hours, explain why						

Water Diverted for Drilling		
Water Source	Amount Taken	Diversion Date & Time
	ig	

Contractor Certification		Certification No
Name of Journeyman responsible for drilling/construction of well		1
UNKNOWN NA DRILLER		Copy of Well report provided to owner
Company Name		Date approval holder signed
TALL PINE DRILLING LTD.		

31/31

Part 2 – Technical Requirements

Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area, and/or manure storage facility(ies)

GENERAL ENVIRONMENTAL INFORMATION

(complete this section for the worst case of the existing facility which is the closest to water bodies or water wells and for each of the proposed facilities)

Facility description / name *(as indicated on site plan)*

Existing: _____ Proposed 1: Feedlot pens
 Proposed 2: Catch basin Proposed 3: _____

Facility and environmental risk information		Facilities				NRCB USE ONLY	
		Existing	Proposed 1	Proposed 2	Proposed 3	Meets requirements	Comments
Flood plain information	What is the elevation of the floor of the lowest manure storage or collection facility above the 1:25 year flood plain or the highest known flood level?	<input checked="" type="checkbox"/> >1 m <input type="checkbox"/> ≤ 1 m	<input checked="" type="checkbox"/> >1 m <input type="checkbox"/> ≤ 1 m	<input checked="" type="checkbox"/> >1 m <input type="checkbox"/> ≤ 1 m	<input type="checkbox"/> > 1 m <input type="checkbox"/> ≤ 1 m	<input checked="" type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> YES with exemption	not in known flood plain
Surface water information	How many springs are within 100 m of the manure storage facility or manure collection area?	0	0	0		<input checked="" type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> YES with exemption	none observed or reported
	How many water wells are within 100 m of the manure storage facility or manure collection area?	0	0	0		<input type="checkbox"/> YES <input type="checkbox"/> NO <input checked="" type="checkbox"/> YES with exemption	one within 100 m of pens, see TD page 6
	What is the shortest distance from the manure collection or storage facility to a surface water body? (e.g., lake, creek, slough, seasonal)	2630m	2630m	2630m		<input checked="" type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> YES with exemption	> 500 m to intermittent creeks
Groundwater information	What is the depth to the water table?		2.59m	2.59m		<input type="checkbox"/> YES <input type="checkbox"/> NO <input checked="" type="checkbox"/> YES with exemption	see Decision Summary RA21043
	What is the depth to the groundwater resource/aquifer you draw water from?	64m	64m	64m		<input checked="" type="checkbox"/> YES <input type="checkbox"/> NO <input type="checkbox"/> YES with exemption	15.2 m based on AEP water wells 93973 and 286659

Additional information (attach supporting information, e.g. borehole logs, records, etc. you consider relevant to your application)



NRCB USE ONLY
ENVIRONMENTAL RISK SCREENING INFORMATION

ERST for proposed facilities

Facility	Groundwater score	Surface water score	File number
Pens	Low	Low	RA21043
Catch basin	Low	Low	RA21043

ERST for existing facilities

Facility	Groundwater score	Surface water score	File number
As noted on page 2 the remaining swine barn will not be used to collect or store manure, for this reason I did not risk screen this facility.			

ERST related comments:

NRCB USE ONLY

WATER WELL AND SURFACE WATER INFORMATION

Well IDs: 286659 and 288714

Surface water related concerns from directly affected parties or referral agencies: YES NO

Groundwater related concerns from directly affected parties or referral agencies: YES NO

Water wells N/A

If applicable, exemption for 100 m distance requirements applied: YES NO Condition required: YES NO

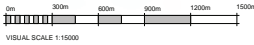
Surface water N/A

If applicable, exemption for 30 m distance requirements applied: YES NO Condition required: YES NO

Water Well Exemption Screening Tool N/A

Water Well ID	Preliminary Screening Score	Secondary Screening Score	Facility
286659	35 (continue)	7 (exemption more likely)	Pens

Groundwater or surface water related comments:



NO	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
B	ISSUED FOR PRELIMINARY REVIEW	2021-07-22
C	ISSUED FOR PRELIMINARY REVIEW	2021-08-30
D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
G	ISSUED FOR ENGINEERING REVIEW	2021-09-31
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22
I	REVISED CATCH BASIN	2021-10-12
J	REVISED CATCH BASIN	2022-01-31
K	REVISED CATCH BASIN	2022-02-09

Double T Cattle Feedlot

Ponoka County, Alberta

15-15-42-25 W4



Stamp:
PRELIMINARY
NOT FOR CONSTRUCTION

aggregate design studio ltd.
(403) 885-5525 P.O. BOX 1690 BLACKFALDS, AB, T0M 0J0

CLIENT SCALE DRAWING
THIS IS A PRELIMINARY DRAWING AND SHALL NOT BE REPRODUCED OR REVISED WITHOUT THE WRITTEN CONSENT OF THE CONSULTANT.
ALL INFORMATION CONTAINED HEREIN IS UNCLASSIFIED AND IS AVAILABLE TO THE PUBLIC. ANY INFORMATION CONTAINED HEREIN IS UNCLASSIFIED AND IS AVAILABLE TO THE PUBLIC. ANY INFORMATION CONTAINED HEREIN IS UNCLASSIFIED AND IS AVAILABLE TO THE PUBLIC.

Neighbour Plan

A1.0

Project number:	A20585
Date:	2022-02-09 12:47:11 PM
Drawn by:	JC
Checked by:	JC
Scale:	As indicated

Part 2 – Technical Requirements

Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area, and/or manure storage facility(ies)



DISTANCE OF ANY MANURE STORAGE FACILITY (EXISTING OR PROPOSED) TO NEIGHBOURING RESIDENCES

Neighbour name(s)	Legal land description	Distance (m)	NRCB USE ONLY				
			Zoning (LUB) category	MDS category (1-4)	Distance (m)	Waiver attached (if required)	Meets regulations
Curtis and Lorraine Tonneson	05-14-42-25 W4	1058	CRH	1	1,045	n/a	yes
Klaus Van Der Molen	04-22-42-25 W4	1047	CRH	1	1,030	n/a	yes
Don and Monica Brennan	12-14-42-25 W4	665	A	1	670	n/a	yes
Brad and Sarah Overreem	02-15-42-25 W4	1160	A	1	1,200	n/a	yes
Sipke and Margriet Dijkstra	15-22-42-25 W4	1370	A	1	1,280	n/a	yes

LAND BASE FOR MANURE AND COMPOST APPLICATION (complete only if an increase in livestock or manure production will occur)

Name of land owner(s)*	Legal land description	Usable area** (ha)	Soil zone ***	NRCB USE ONLY	
				Usable area (ha)	Agreement attached (if required)
Double T	section 15-42-25 w4	242.8	Black	198	
Double T	sw-14-42-25 w4	56.7	Black	50	
Morsan	north 20-42-25 w4	93.1	Black	81	yes
	SE 25-42-25 W4	140ac/57 ha		45	yes
	SE 12-43-25 W4	109 ac/44 ha		41	yes
			Total		

* If you are **not** the registered landowner, you must attach copies of land use agreements signed by all landowners.

** Available manure spreading area (excluding setback areas from residences, common bodies of water, water wells, etc. as identified in Agdex 096-5 [Manure Spreading Regulations](#))

*** Brown, dark brown, black, grey wooded, or irrigated

Additional information (attach any additional information as required)

Last updated February 26, 2021

Ponoka County's Land use bylaw zones and the applicable MDS categories are discussed in Decision Summary RA21043.

5/21

NRCB USE ONLY

MINIMUM DISTANCE SEPARATION

Methods used to determine distance (if applicable): scaled air photo, 2021 from Google Earth

Margin of error (if applicable): _____

Requirements (m): Category 1: 659 Category 2: 878 Category 3: 1,098 Category 4: 1,756

Technology factor: YES NO

Expansion factor: YES NO

MDS related concerns from directly affected parties or referral agencies: YES NO

The MDS related concerns are discussed in Decision Summary RA21043.

LAND BASE FOR MANURE AND COMPOST APPLICATION

Land base required: 351 ha (black)

Land base listed: 493.6 ha

Area not suitable: 78.6 ha

Available area: 415 ha

Requirement met: YES NO

Land spreading agreements required: YES NO

Manure management plan: YES NO If yes, plan is attached:

PLANS

Submitted and attached construction plans: YES NO

Submitted aerial photos: YES NO

Submitted photos: YES NO

GRANDFATHERING

Already completed: YES NO N/A

If already completed, see Decision Summary RA02053

Manure Spreading Agreement

This agreement is between _____ Double T Cattle _____, manure producer, and

_____ Morsan Farms _____ Manure receiver.

Length of agreement: This agreement is valid for a time period of _____ 3 years _____ (minimum of three years)

Legal land location	Soil type ¹	Acres suitable for manure spreading ²
N 20-42-25-W4	Black	230 acres 81 ha available
SE-25-42-25-W4	Black	140 acres 45 ha available
SE-12-43-25-W4	Black	109 acres 41 ha available

¹ Soil type choices: Dark brown and brown, Grey wooded, Black, Irrigated.

² Land within required setbacks from water bodies, water wells, residences, etc. is not to be included.

Other comments:

Manure producer (confined Feeding Operation) Legal Land Location _____

May 12 2022 [Signature] Joe Thaler. _____
 Date of signing Signature Print name Corporate name(if appl)

Manure Receiver – Landowner(s)³

May 12 2022 [Signature] Morsan Thaler. _____
 Date of signing Signature Print name Corporate name(if appl)

_____ _____ _____ _____
 Date of signing Signature Print name Corporate name(if appl)

³ All registered owners of land, or authorized signing authorities must sign.

Catch Basin Dimension Calculator

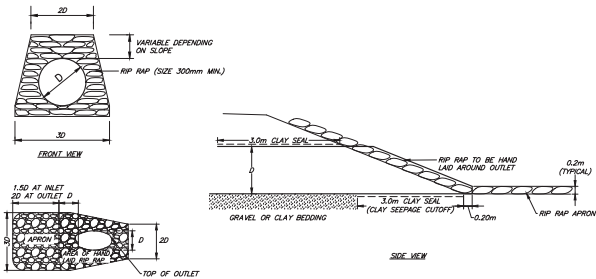
For more information on runoff control catch basin design considerations including liner options, catch basin protection, etc., check out the catch basin [factbook](#).

Name: Double T Cattle Feed Lot
 Land Location: Ponoka County

Estimating Runoff Potential

Area	Length (m)	Width (m)	Paved?	Area (m ²)
1	229	108	YES	24742.00
2	48	33	YES	1584.00
3	48	107	YES	5136.00
4			NO	1.0
Total Area				31469.00

Estimation of water runoff to be collected in the catch basin:
 3069.52 m³
 214341 g³
 1335107 Imp. Gal



PROPOSED	EXISTING
GRADING	890.00
CONTOURS	X100.000
ELEVATIONS	X100.000
SLOPES	0.80%
SHALL/DITCH SLOPES	
DEEP UTILITIES	
SANITARY SEWER	
STORM SEWER	
CATCHBASIN/CATCHBASIN MANHOLE	
CULVERT	
HYDRANT	
REDUCER	
TEE	
VALVE	
PLUG OR CAP	
SHALLOW UTILITIES	
GAS LINE	
BURIED POWER LINE	
SHAW LINE	
TELUUS LINE	
OVERHEAD POWER	
FIBER OPTIC LINE	
TRANSFORMER	
TELUUS PEDESTAL	
SHAW PEDESTAL	
TELUUS/SHAW PEDESTAL	
3 PARTY PEDESTAL	
POWER POLE	
LIGHT POLE	

PROPOSED	EXISTING
ROADWAYS	
CENTERLINE	LOG
CONCRETE CURB	LOG
EDGE OF ASPHALT	BOC
EDGE OF GRAVEL	
DITCH BOTTOM	
SIGNS	
LEGAL	
PROPERTY LINE	
EASEMENT	
MISCELLANEOUS	
FENCE	
TIE IN BOUNDARY	
CATCH BASIN	
CATCHMENT BOUNDARY	

#	DATE	DESCRIPTION	BY
1	2021-10-08	ISSUED FOR REVIEW	JMR
2	2021-11-09	ISSUED FOR REVIEW - REVISED CATCHBASIN	JMR
3	2022-01-20	ISSUED FOR REVIEW - REVISED CATCHBASIN	JMR
4	2022-02-03	ISSUED FOR REVIEW - REVISED CATCHBASIN 2	JMR
5	2022-02-23	ISSUED FOR REVIEW	PSL

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 #202, 4708 50th AVENUE, RED DEER, ALBERTA PRC 4E3-340-3022

NOT FOR CONSTRUCTION

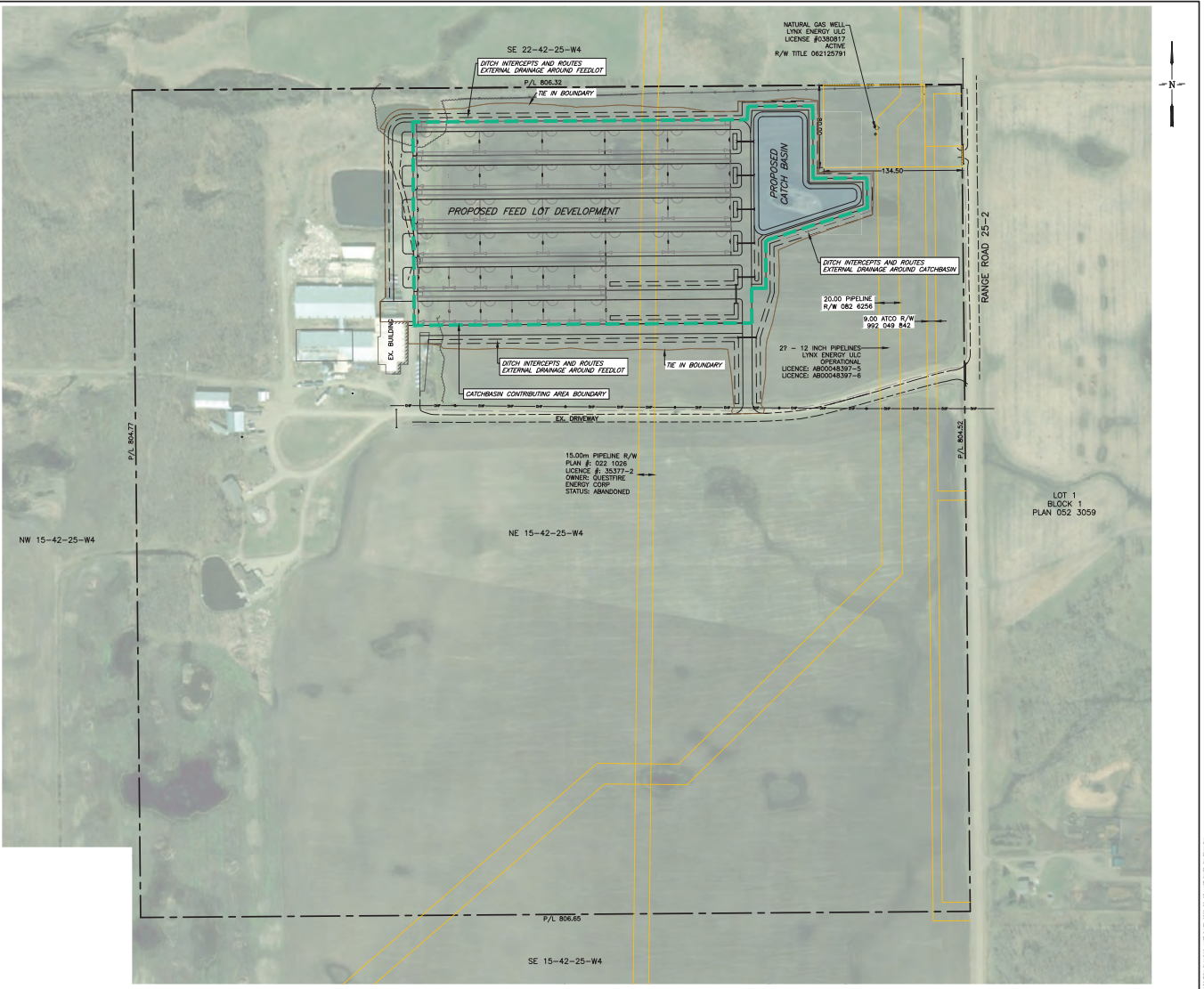
CLIENT: **EAGLE BUILDERS**
 PROJECT: **DOUBLE T CATTLE FEEDLOT**
 DRAWING TITLE: **SITE LOCATION & RUNOFF CONTROL CATCH BASIN PLAN**

PROJECT NO.: 5457 DRAWING: TMW
 NE 15-42-25-84
 PONOKA COUNTY, AB
 DESIGNED: JMR
 REVIEWED: FQ
 PLOTTED: 2022-02-23
 SHEET No.: 1/2

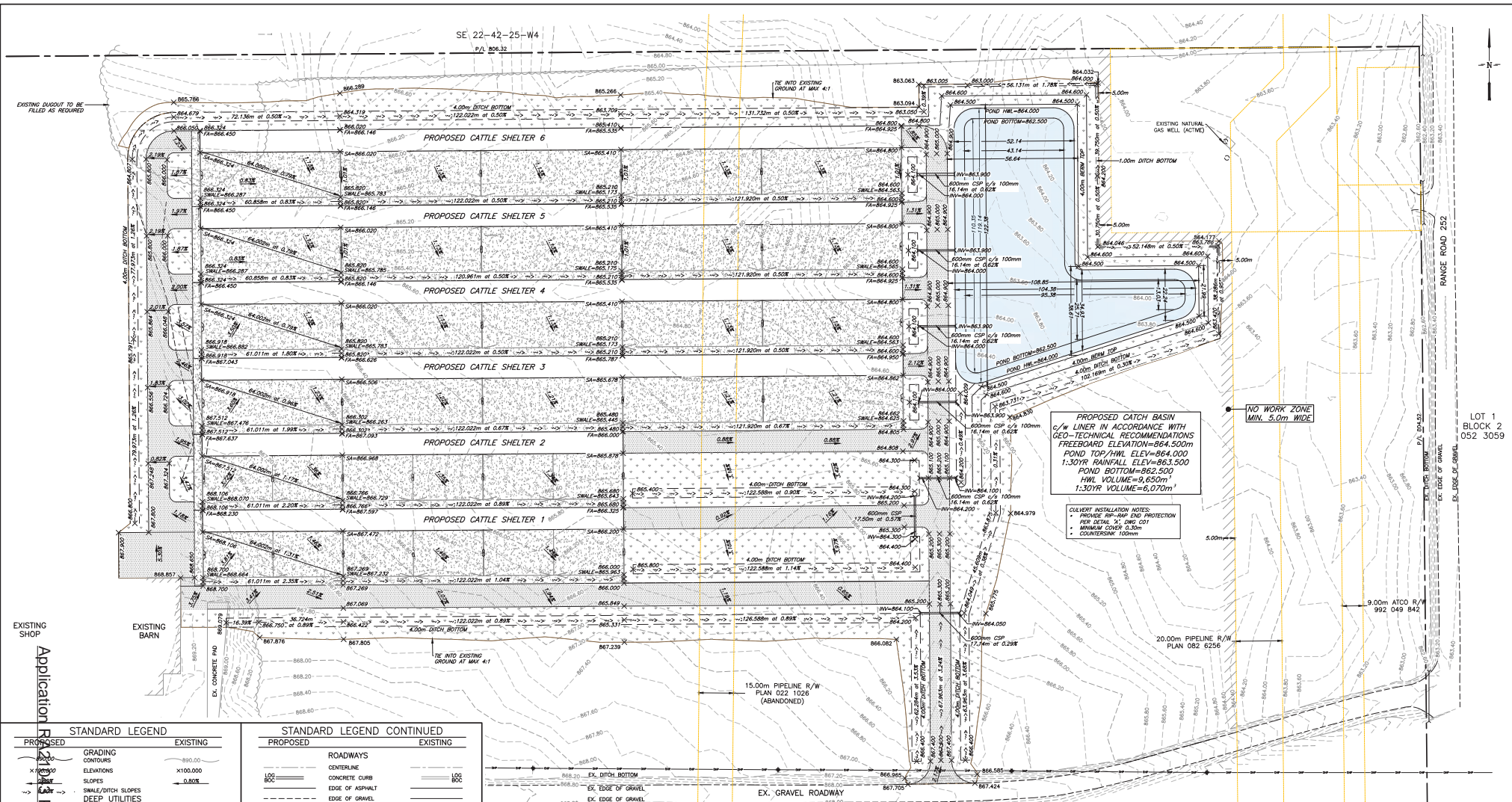
SCALE: 1:12000
 25 50 100

NOT TO SCALE UNLESS NOTED OTHERWISE

C01



SE 22-42-25-W4
FA 806.32



PROPOSED CATCH BASIN
G/W LINER IN ACCORDANCE WITH
GEO-TECHNICAL RECOMMENDATIONS
FREESBOARD ELEVATION=864.500m
POND TOP/HWL ELEV=864.000
1.30YR RAINFALL ELEV=863.500
POND BOTTOM=862.500
HWL VOLUME=9,650m³
1.30YR VOLUME=6,070m³

CULVERT INSTALLATION NOTES:
• PROVIDE RIP-RAP END PROTECTION
• PER DETAIL 31, DWG C01
• MINIMUM COVER 0.30m
• COUNTERSINK 100mm

NO WORK ZONE
MIN. 5.0m WIDE

STANDARD LEGEND

PROPOSED	EXISTING
GRADING CONTOURS	GRADING CONTOURS
ELEVATIONS	ELEVATIONS
SLOPES	SLOPES
SHALLOW DITCH SLOPES	SHALLOW DITCH SLOPES
DEEP UTILITIES	DEEP UTILITIES
SANITARY SEWER	SANITARY SEWER
STORM SEWER	STORM SEWER
CATCHBASIN/CATCHBASIN MANHOLE	CATCHBASIN/CATCHBASIN MANHOLE
CULVERT	CULVERT
HYDRANT	HYDRANT
RENDER	RENDER
TIE	TIE
VALVE	VALVE
PLUG OR CAP	PLUG OR CAP
SHALLOW UTILITIES	SHALLOW UTILITIES
GAS LINE	GAS LINE
BURIED POWER LINE	BURIED POWER LINE
SHAW LINE	SHAW LINE
TELUS LINE	TELUS LINE
OVERHEAD POWER	OVERHEAD POWER
FIBER OPTIC LINE	FIBER OPTIC LINE
TRANSFORMER	TRANSFORMER
TELUS PEDESTAL	TELUS PEDESTAL
SHAW PEDESTAL	SHAW PEDESTAL
TELUS/SHAW PEDESTAL	TELUS/SHAW PEDESTAL
3 PARTY PEDESTAL	3 PARTY PEDESTAL
POWER POLE	POWER POLE
LIGHT POLE	LIGHT POLE

STANDARD LEGEND CONTINUED

PROPOSED	EXISTING
ROADWAYS	ROADWAYS
CENTERLINE	CENTERLINE
CONCRETE CURB	CONCRETE CURB
EDGE OF ASPHALT	EDGE OF ASPHALT
EDGE OF GRAVEL	EDGE OF GRAVEL
DITCH BOTTOM	DITCH BOTTOM
SIGNS	SIGNS
LEGAL	LEGAL
PROPERTY LINE	PROPERTY LINE
EASEMENT	EASEMENT
DESIGN GROUND ELEVATION	DESIGN GROUND ELEVATION
DESIGN FEED ALLEY ELEVATION	DESIGN FEED ALLEY ELEVATION
DESIGN SWALKER ALLEY ELEVATION	DESIGN SWALKER ALLEY ELEVATION
DESIGN FEED ALLEY ELEVATION	DESIGN FEED ALLEY ELEVATION
FENCE	FENCE
TIE IN BOUNDARY	TIE IN BOUNDARY
CONCRETE	CONCRETE
LANDSCAPING	LANDSCAPING
GRAVEL	GRAVEL
CATCH BASIN	CATCH BASIN
OVERLAND DRAINAGE ROUTE	OVERLAND DRAINAGE ROUTE

- NOTES:**
- ELEVATIONS ARE TO TOP OF ASPHALT OR FINISHED GROUND UNLESS OTHERWISE NOTED.
 - EXISTING GRADES:
 - DESIGN GRADES: 0.80%
 - EXISTING GROUND ELEVATION: 0.00
 - DESIGN GROUND ELEVATION: 0.80%
 - DESIGN FEED ALLEY ELEVATION: 0.00
 - DESIGN SWALKER ALLEY ELEVATION: 0.00
 - DESIGN FEED ALLEY ELEVATION: 0.00
 - CONSTRUCTION TO BE COMPLETED IN ACCORDANCE WITH PONDOKA COUNTY ENGINEERING STANDARDS.
 - TOPOGRAPHIC SURVEY COMPLETED BY AL-TERRA ENGINEERING (RED DEER) ON JULY 22, 2021. SITE PLAN SUPPLIED BY OTHERS.
 - CONTRACTOR RESPONSIBLE FOR ALL UTILITY LOCATES PRIOR TO CONSTRUCTION.

NOTES FOR BUILDINGS:

- REFER TO STRUCTURAL & ARCHITECTURAL DRAWINGS FOR ANY GRADING WITHIN THE BUILDING FOOTPRINT.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF ALL SITE FEATURES AND AMENITIES.
- FOUNDATION WALL FOR CATTLE SHELTERS TO BE EXPOSED ON THE NORTH SIDE OF FEED ALLEY PER BUILDING CROSS SECTIONS. HEIGHT OF EXPOSURE = FEED ALLEY ELEVATION - ADJACENT SWALE EDGE ELEVATION. REFER TO ARCHITECTURAL DRAWINGS FOR DETAILS.

#	DATE	DESCRIPTION	BY	CHK
1	2021-09-20	ISSUED FOR REVIEW	JMR	JMR
2	2021-10-08	ISSUED FOR REVIEW - REVISED CATCHBASIN	JMR	JMR
3	2022-01-20	ISSUED FOR REVIEW - REVISED CATCHBASIN	JMR	JMR
4	2022-02-01	ISSUED FOR REVIEW - REVISED CATCHBASIN 2	JMR	JMR
5	2022-02-22	ISSUED FOR REVIEW	JMR	JMR

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NOT FOR CONSTRUCTION

CLIENT: **EAGLE BUILDERS**
 PROJECT: **DOUBLE T CATTLE FEEDLOT**
 DRAWING TITLE: **SITE GRADING PLAN**

PROJECT NO.: 5457
 NE 15-42-25-W4
 PONDOKA COUNTY, AB

DESIGNED: JMR
 REVIEWED: FQ
 PLOTTED: 2018-12-13
 SHEET NO.: 2/2

SCALE: 1:1750
 10 20 40

C02

Layfield's HDPE geomembranes are manufactured to meet GRI-GM13 Standard Specification and have been extensively used in a variety of containment applications. HDPE geomembranes have low permeability, good ultra violet resistance, and excellent chemical resistance. HDPE 60 geomembrane is a field assembled lining material that must be installed by trained installers. HDPE is used in a multitude of applications such as a landfill liner, a pond liner, and in water containment projects.

Geomembrane Properties			
Property	Test Method	Values	Testing Frequency
Thickness (min. avg)	ASTM D5994	1.50 mm	
Thickness, lowest individual for 8 out of 10 values		1.53 mm	Every Roll
Sheet Density (min.)	ASTM D792	0.940 g/cc	Every 90,000 kg
Tensile Properties (min. avg.)			
Break Strength		40 kN/m	
Break Elongation (calculated at gage length of 2 in.)	ASTM D6693	700%	Every 9,000 kg
Yield Strength	Type IV	22 kN/m	
Yield Elongation (calculated at gage length of 1.3 in.)		12%	
Tear Resistance (min. avg)	ASTM D1004	187 N	Every 20,000 kg
Puncture Resistance (min. avg)	ASTM D4833	480 N	Every 20,000 kg
Stress Crack Resistance (min.)	ASTM D5397 (Appendix X1)	500 hrs	Once every two resin railcars
High Pressure Oxidative Induction Time (min.)	ASTM D5885	400 minutes	Every 90,000 kg
Carbon Black Content	ASTM D4218	2.0 - 3.0%	Every 9,000 kg
Carbon Black Dispersion	ASTM D5596	9 out of 10 in CAT 1 or 2; 1 in CAT 3	Every 20,000 kg
Oven Ageing at 85° C	ASTM D5721		
% retained HPOIT after 90 days (min.)	ASTM D5885	80%	Once per formulation
UV Resistance	ASTM D7238		
% retained HPOIT after 1600 hrs (min.)	ASTM D5885	50%	Once per formulation
Resin Properties			
Resin Density (minimum)	ASTM D792	0.932 g/cc	Once per railcar
Resin Melt Index at 2.16 Kg/190°C (max.)	ASTM D1238	1.0 g/10 min	Once per railcar
Thermally Bonded Seam Strength (as per GRI-GM19)			
Hot Wedge Seams - Shear Strength (min.)	ASTM D6392	525 N/25mm	Not Applicable
Hot Wedge Seams - Peel Strength (min.)	ASTM D6392	398 N/25mm	Not Applicable
Extrusion Fillet Seams – Shear Strength (min.)	ASTM D6392	525 N/25mm	Not Applicable
Extrusion Fillet Seams – Peel Strength	ASTM D6392	340 N/25mm	Not Applicable
Typical Roll Dimensions (Rolls dimensions may vary ± 1%)			
Roll Width		6.86 meters	
Roll Length		158.5 meters	

Latest product specifications are available on Layfield Group website www.layfieldgroup.com

Disclaimer: Layfield disclaims any and all express, implied, or statutory standards, warranties or guarantees, including without limitation any implied warranty as to merchantability or fitness for a particular purpose or arising from a course of dealing or usage of trade as to any equipment, materials, or information furnished herewith. This document should not be construed as engineering advice.

Part 2 – Technical Requirements

Application under the Agricultural Operation Practices Act for a confined feeding operation, manure collection area and/or manure storage facility(ies)

LIQUID MANURE STORAGE: Synthetic liner

(complete a copy of this section for EACH proposed liquid manure storage facility with a synthetic liner)

Facility description / name (as indicated on site plan) **1.** Feedlot Catch Basin
2. _____

Manure storage capacity (use one row in the table for EACH cell of the synthetic lined storage, attach additional pages if you require more rows)

	Length (m)	Width (m)	Total depth (m)	Depth below ground level (m)	Slope run:rise			NRCB USE ONLY	
					Inside end walls	Inside side walls	Outside walls	Calculated storage capacity (excl. 0.5 m freeboard) (m ³)	Filled in lower ¼? Y/N
1.	95	110	2	1.15*	3:1	3:1	4:1	9,650	no
2.	See drawings attached for slope and volumes. 56.64 x 122.38								
TOTAL CAPACITY									

Surface water control systems

Describe the run-on and runoff control system

Berms elevated above surrounding landscape with 0.5m free-board.

* the southwest corner will be approximately 2.1 m below ground, the water table was observed to be as shallow as 2.47 m below ground surface in fall 2021. A condition will be added to the permit, see discussion in Decision Summary RA21043.

Sealing

Describe sealing practices for piping, etc. that penetrates the liner

Pipe boot welded to liner and clamped to pipe.

NRCB USE ONLY

Requirements met: YES NO

Liner protection

Describe how the inside walls, bottom and outside walls are protected from erosion

Concrete pad will be poured under pipe fill area. Vegetation on exterior berm slope.

Describe how the physical integrity of the liner will be maintained from other damage

Care and caution will be used to prevent damage to liner.

NRCB USE ONLY

Requirements met: YES NO

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Part 2 – Technical Requirements

Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area and/or manure storage facility(ies)

LIQUID MANURE STORAGE: Synthetic liner (cont.)

Synthetic liner details

Provide synthetic liner material details
 60mil HD liner. Spec sheet attached.

Additional information *(attach copies of design/engineering reports)*

NRCB USE ONLY

Requirements met: YES NO
 Condition required: YES NO
 Report attached: YES NO

NRCB USE ONLY

Catch basin
 Liquid manure storage volume calculator attached: YES NO
 Depth to water table: 2.47 m
 Depth to uppermost groundwater resource: 15.2 m
 Requirements met: YES NO
 Requirements met: YES NO
 As noted previously, a condition will be added to the permit to address water table separation.

ERST completed: see ERST page for details

Surface water control systems
 Requirements met: YES NO Details/comments:

Synthetic liner requirements
 Leakage detection system required: YES NO If yes, please explain why.

Construction plans approved by professional engineer: YES NO
 Will liner be installed by manufacturer approved contractor and qualified third party?: YES NO
 Preparation of liner bed (comments):
 A condition will be added requiring the liner to be installed by a qualified third party and that a completion report be submitted.

Condition required: YES NO

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NRCB USE ONLY	
RUNOFF CONTROL CATCH BASIN CAPACITY SUMMARY (if applicable)	
Facility 1	feedlot
Name / description	catch basin
Capacity	9,650 m ³ *
Facility 2	
Name / description	
Capacity	
Facility 3	
Name / description	
Capacity	
Facility 4	
Name / description	
Capacity	
TOTAL CAPACITY	9,650 m³
RUNOFF VOLUME FROM CONTRIBUTING AREAS	5,901 m³
MEETS AOPA RUNOFF CONTROL VOLUME REQUIREMENTS	<input checked="" type="checkbox"/> YES <input type="checkbox"/> NO

* the stated catch basin capacity is reasonable, see page 76 below.

The approximate catchment area for the catch basin is 81,600 m². Of this, approximately 62,000m² will be either paved by concrete, covered by roofs, or have a synthetic liner.

The 1:30 year rainfall event in the the Standards and Administration regulation for Ponoka is 80 mm.

$$62,000\text{m}^2 \times 0.080 \text{ m} \times 1.0 \text{ (runoff coefficient for "paved" area)} = 4,960 \text{ m}^3$$

$$19,600 \text{ m}^2 \times 0.080 \text{ m} \times 0.6 \text{ (runoff coefficient for unpaved area with 80 mm rainfall)} = 941 \text{ m}^3$$

Based on the above, I would expect there to be approximately 5,901 m³ of runoff directed to the catch basin in a 1:30 year rainfall event.

Part 2 – Technical Requirements

Application under the Agricultural Operation Practices Act for a confined feeding operation, manure collection area and/or manure storage facility(ies)

SOLID MANURE, COMPOST, & COMPOSTING MATERIALS: Barns, feedlots, & storage facilities - Concrete liner

(complete a copy of this section for **EACH** barn, feedlot, and storage facility for solid manure, composting materials, or compost with a concrete liner)

- Facility description / name (as indicated on site plan)
1. Feedlot pens _____
 2. _____

Manure storage capacity

	Length (m)	Width (m)	Depth below grade to the bottom of the liner (m)	NRCB USE ONLY Estimated storage capacity (m ³)
1.	4 x 304.8	32.97	0	
2.	2 x 182.88	32.97	0	
TOTAL CAPACITY				

I plan to use a short-term solid manure storage (STMS) as part of my manure storage and handling plan for this CFO. The AOPA requirements for STMS are set out in the NRCB [Short-Term Solid Manure Storage Requirements Fact Sheet](#).

Surface water control systems

Describe the run-on and runoff control system
Catch basin used

Liner protection

Describe how the physical integrity of the liner will be maintained
Cleaned and inspected periodically

NRCB USE ONLY

Requirements met: YES NO

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Part 2 – Technical Requirements

Application under the *Agricultural Operation Practices Act* for a confined feeding operation, manure collection area and/or manure storage facility(ies)

SOLID MANURE, COMPOST, & COMPOSTING MATERIALS: Barns, feedlots, & storage facilities - Concrete liner (cont.)

Concrete liner details

Concrete thickness 0.152m (6")	Method of sulphate protection: type 10 HS cement
Concrete strength 30 mpa	Concrete reinforcement size and spacing 10M bar at 0.305m on center each way

Concrete requirements can be found in Technical Guideline Agdex 096-93

Guideline minimums:
Solid manure: 25MPa (D)
Solid manure (wet): 30MPa (C)
Method of sulphate protection:
Type 50 or Type 10 with fly ash or equivalent

NRCB USE ONLY

Requirements met: YES NO
Condition required: YES NO
Report attached: YES NO

Additional information *(attach as required)*

NRCB USE ONLY

Nine month manure storage volume requirements met YES YES With STMS NO
Depth to water table: 2.47 m Requirements met: YES NO
Depth to Uppermost groundwater resource: 15.2 m Requirements met: YES NO
ERST completed: see ERST page for details

Surface water control systems

Requirements met: YES NO Details/comments:

Concrete liner details

The proposed concrete and rebar meet the requirements of Agdex 096-93. Despite this, the large concrete pads can be prone to cracking. Accordingly, conditions will be added to the permit requiring 1. the pad be constructed with an equivalent concrete liner with rebar as proposed and 2. the pad/pens to have adequate sealed expansion joints or crack control.

Leakage detection system required: YES NO If yes, please explain why.

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design studio ltd.



Double T Cattle

SYMBOL LEGEND

CONSTRUCTION REFERENCE	CONSTRUCTION TAG
FINISH REFERENCE	EXTERIOR FINISH TAG
GRID NUMBER	GRID BUBBLE
DOOR REFERENCE	DOOR TAG
ROOM NUMBER	ROOM TAG
DESCRIPTION	ELEVATION DATUM
DRAWING NUMBER	WALL SECTION CALLOUT
SHEET NUMBER	BUILDING SECTION CALLOUT
DRAWING NUMBER	ELEVATION CALLOUT
SHEET NUMBER	WINDOW TAG
ELEVATION	ELEVATION TAG
DRAWING TITLE	DRAWING TITLE

DRAWING INDEX

Sheet Number	Sheet Name	Current Revision
A0.0	Index	K
A1.0	Neighborhood Plan	K
A1.1	Site Plan	K
A2.0	Floor Plan Overall	H
A2.1	Floor Plan Enlarged	H
A2.2	Roof Plan Overall	G
A4.1	Building Sections	F
A4.2	Building Sections	F
A4.3	Wall Sections	F
A5.0	Architectural Details	F
A5.1	Architectural Details	F

Ponoka County, Alberta

Feedlot

Application RA21043 Page 32 of 45

NO	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
B	ISSUED FOR PRELIMINARY REVIEW	2021-07-22
C	ISSUED FOR PRELIMINARY REVIEW	2021-08-20
D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
E	ISSUED FOR PRELIMINARY REVIEW	2021-08-25
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
G	ISSUED FOR ENGINEERING REVIEW	2021-09-21
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22
I	REVISED CATCH BASIN	2021-09-22
J	REVISED CATCH BASIN	2022-01-31
K	REVISED CATCH BASIN	2022-02-09

Double T Cattle
Feedlot

Ponoka County, Alberta

15-15-42-25 W4



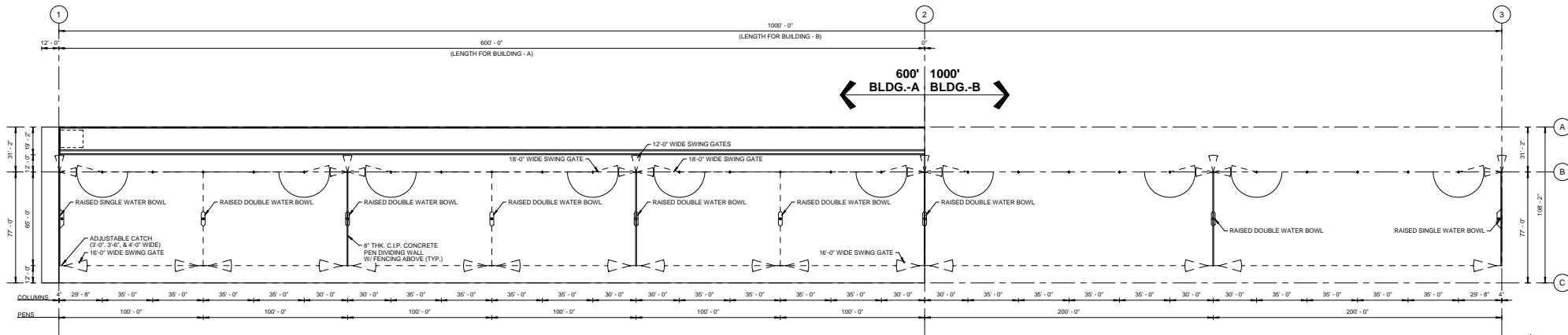
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Index
A0.0

Project number:	A20585
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1 Overall Floor Plan
A2.0
1/32" = 1'-0"



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NO.	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
B	ISSUED FOR PRELIMINARY REVIEW	2021-07-22
C	ISSUED FOR PRELIMINARY REVIEW	2021-08-20
D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
E	ISSUED FOR PRELIMINARY REVIEW	2021-08-25
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
G	ISSUED FOR ENGINEERING REVIEW	2021-09-21
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22

Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4

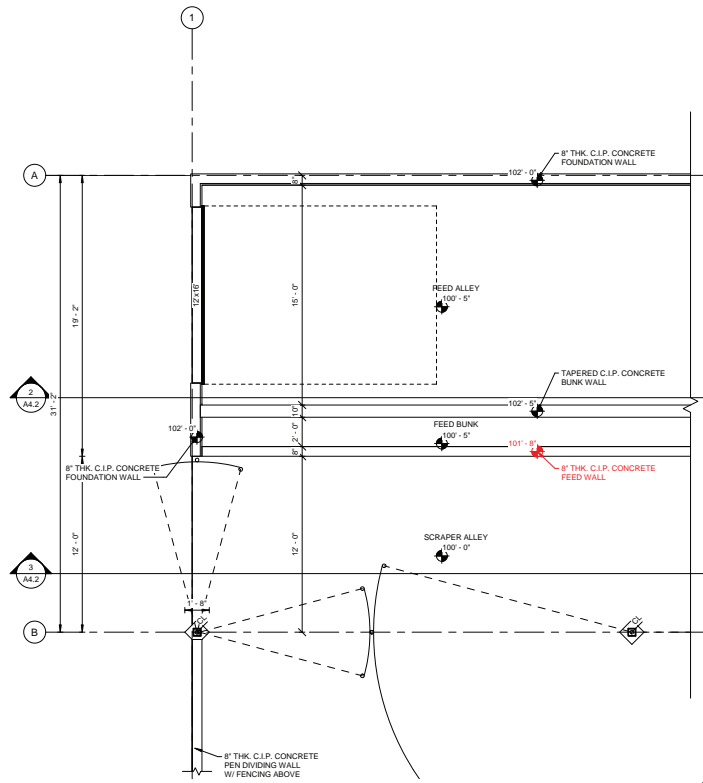


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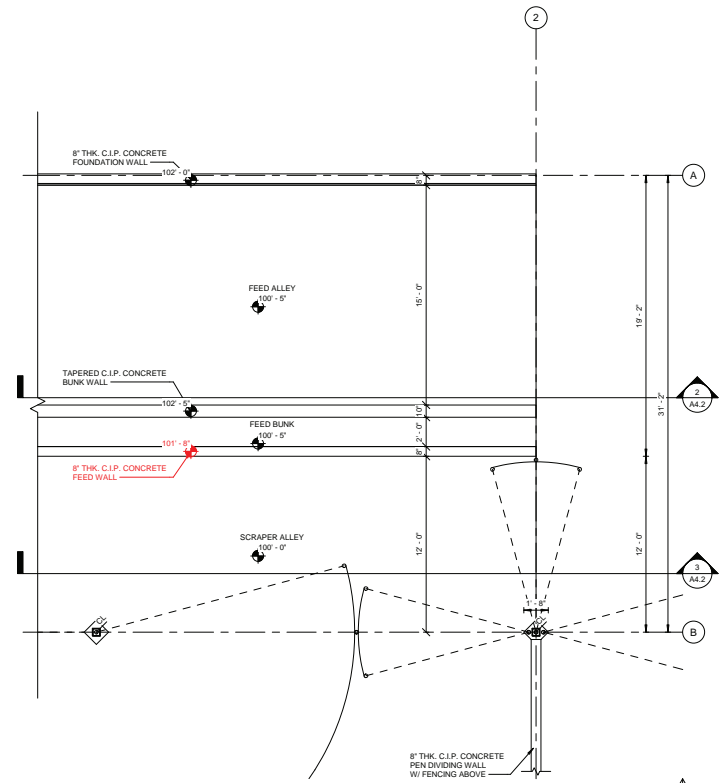
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Floor Plan Overall
A2.0

Project number: **A20585**
Date: **2022-02-09 12:48:04 PM**
Drawn by: **JC**
Checked by: **JC**
Scale: **As indicated**



3 Enlarged Floor Plan - West
A2.1 1/4" = 1'-0"



2 Enlarged Floor Plan - East
A2.1 1/4" = 1'-0"



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NO.	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
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D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22

Double T Cattle Feedlot

Ponoka County, Alberta

15-15-42-25 W4



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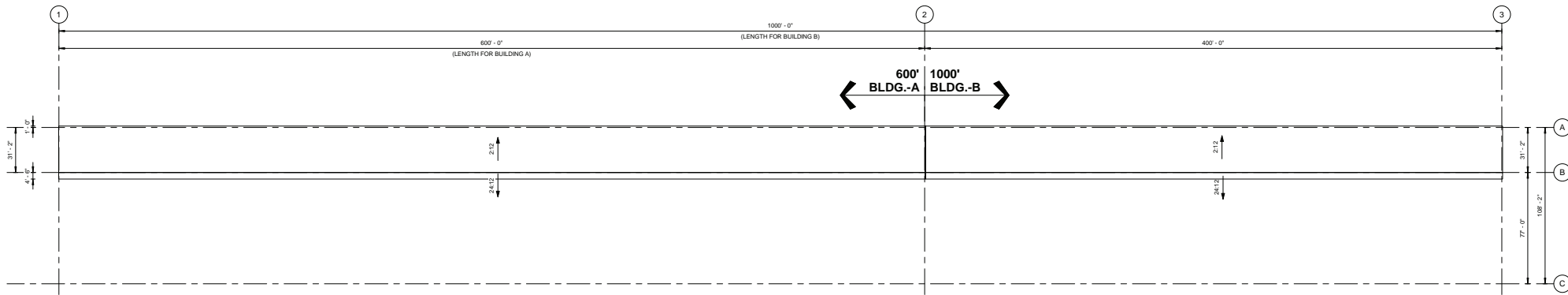
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Floor Plan Enlarged

A2.1

Project number:	A20585
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Drawn by:	JC
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Scale:	As indicated



4 Overall Roof Plan
A2.2
1/32" = 1'-0"



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NO.	DESCRIPTION	DATE
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
G	ISSUED FOR ENGINEERING REVIEW	2021-09-21

Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4

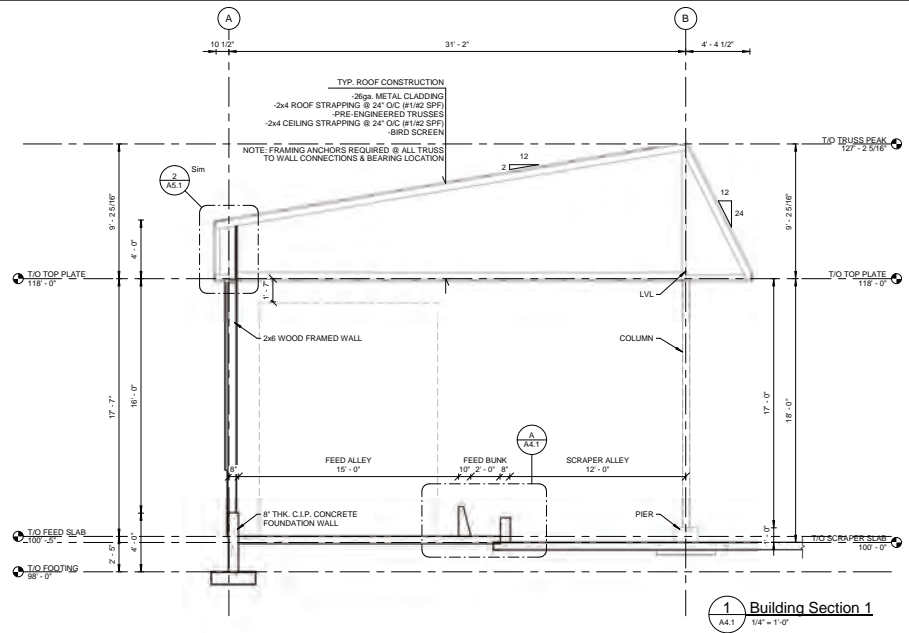


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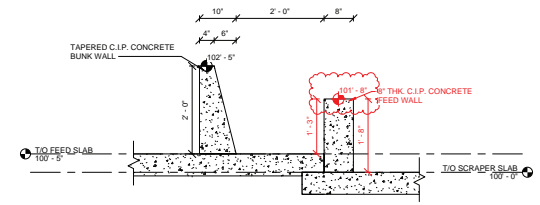
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Roof Plan Overall
A2.2

Project number:	A20585
Date:	2022-02-09 12:48:04 PM
Drawn by:	JC
Checked by:	JC
Scale:	As indicated



1 Building Section 1
A4.1 1/4" = 1'-0"



A Feed Bunk Section
A4.1 3/4" = 1'-0"

NO.	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
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D	ISSUED FOR PRELIMINARY REVIEW	2021-08-24
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30

Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4

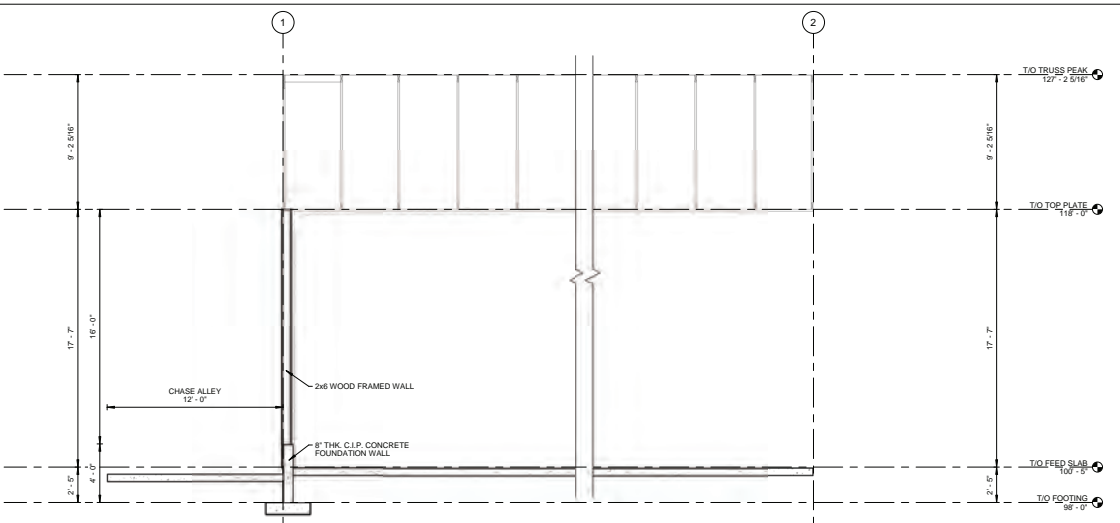


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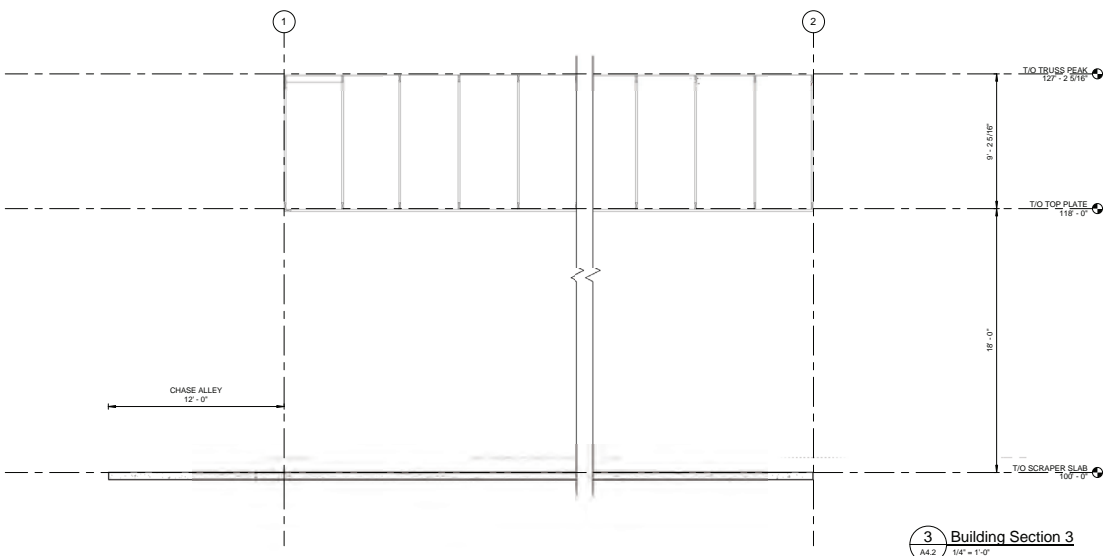
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Building Sections
A4.1

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Date:	2022-02-09 12:48:05 PM
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2 Building Section 2
A4.2 1/4" = 1'-0"



3 Building Section 3
A4.2 1/4" = 1'-0"

NO.	DESCRIPTION	DATE
A	ISSUED FOR PRELIMINARY REVIEW	2021-07-19
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F	ISSUED FOR ENGINEERING REVIEW	2021-08-30
H	ISSUED FOR ENGINEERING REVIEW	2021-09-22

Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4

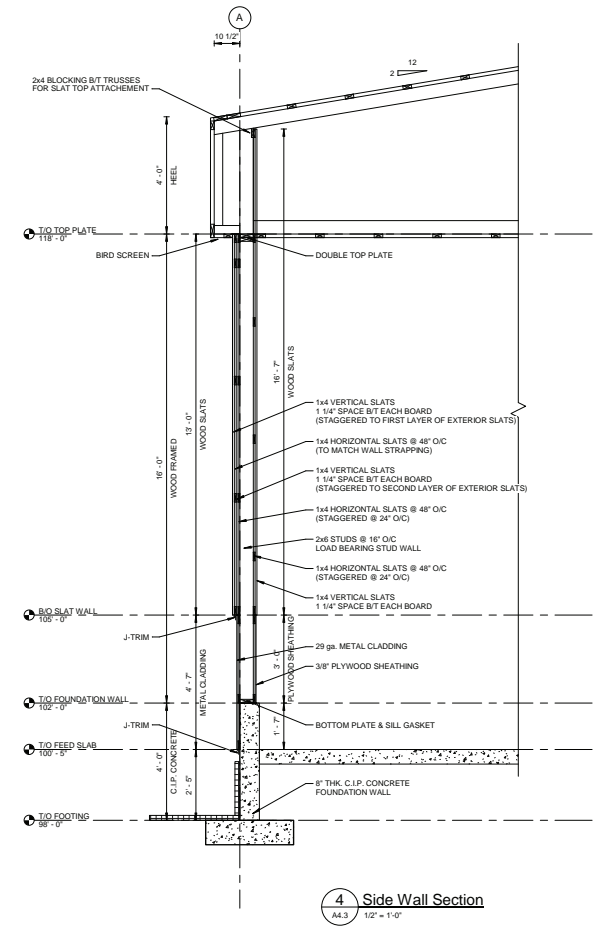
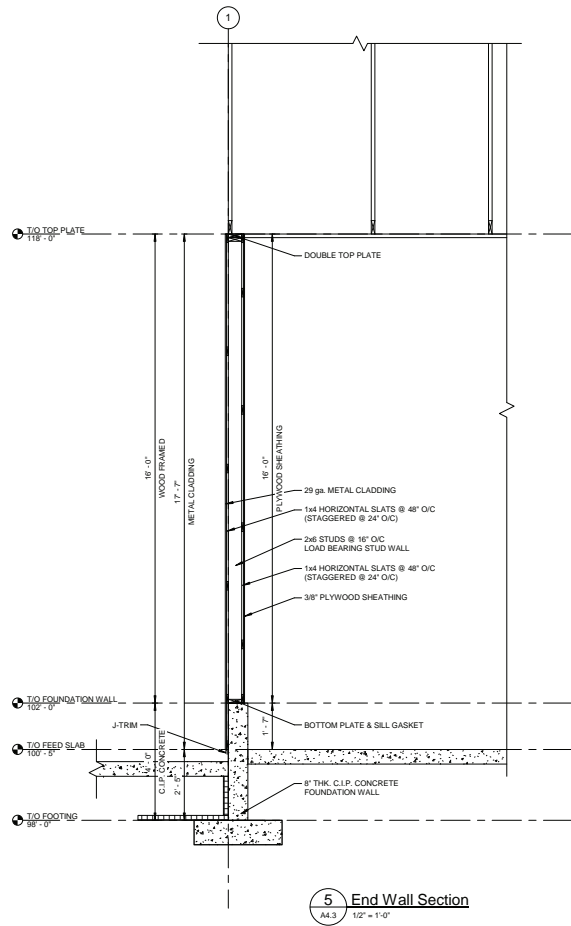


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Building Sections
A4.2

Project number: **A20585**
Date: **2022-02-09 12:48:07 PM**
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Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4



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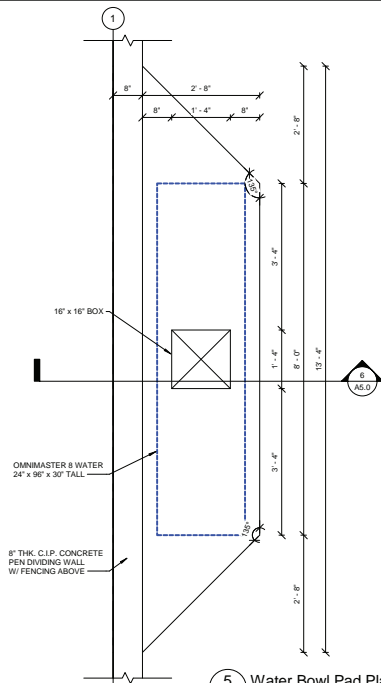
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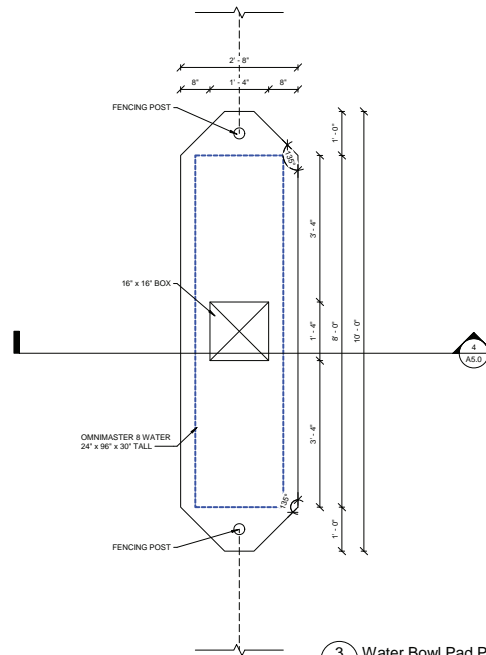
Wall Sections

A4.3

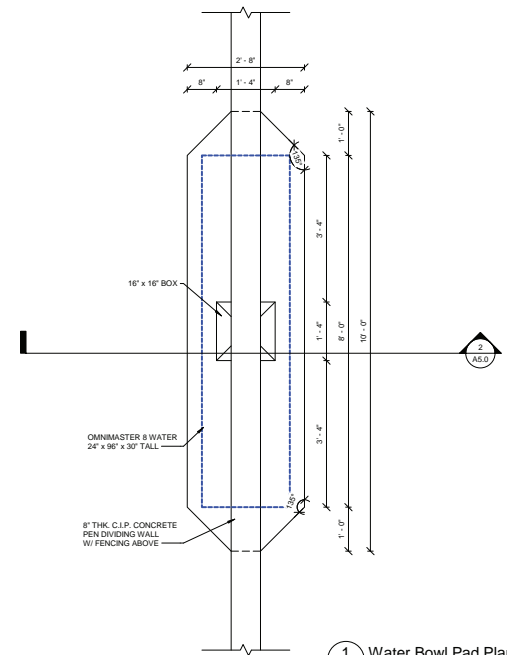
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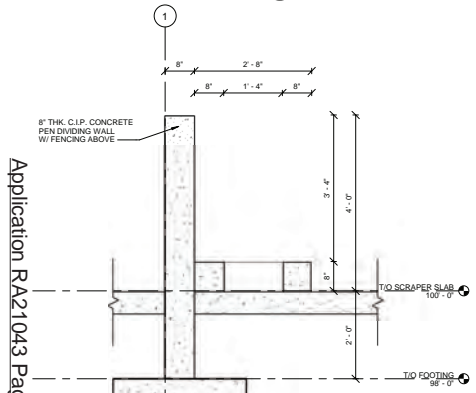
5 Water Bowl Pad Plan - Single
A5.0 3/4" = 1'-0"



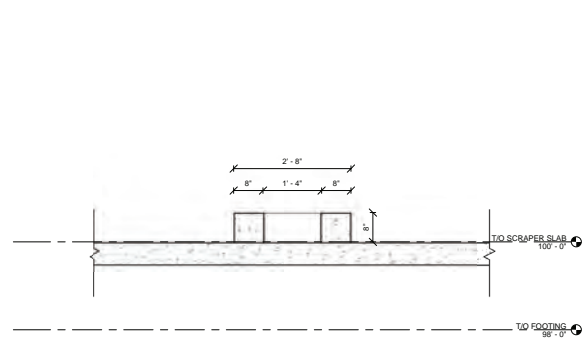
3 Water Bowl Pad Plan - Double @ Fence
A5.0 3/4" = 1'-0"



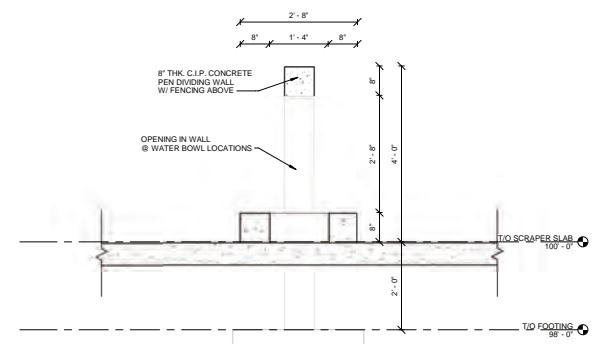
1 Water Bowl Pad Plan - Double @ Wall
A5.0 3/4" = 1'-0"



6 Water Bowl Pad Section - Single
A5.0 3/4" = 1'-0"



4 Water Bowl Pad Section - Double @ Fence
A5.0 3/4" = 1'-0"



2 Water Bowl Pad Section - Double @ Wall
A5.0 3/4" = 1'-0"

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Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4

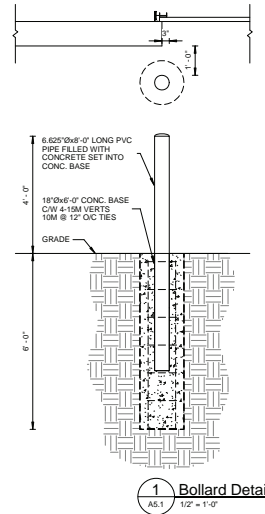


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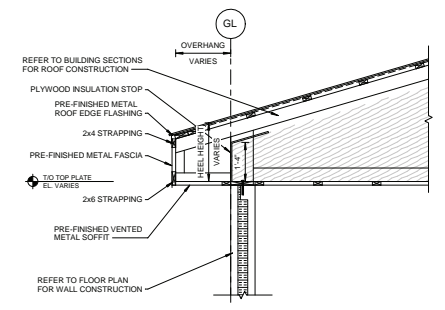
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Architectural Details
A5.0

Project number:	A20585
Date:	2022-02-09 12:48:09 PM
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1 Bollard Detail
A5.1
1/2" = 1'-0"



2 Soffit Detail
A5.1
1/2" = 1'-0"

NO.	DESCRIPTION	DATE
F	ISSUED FOR ENGINEERING REVIEW	2021-08-30

Double T Cattle Feedlot
Ponoka County, Alberta
15-15-42-25 W4



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Architectural Details
A5.1

Project number:	A20585
Date:	2022-02-09 12:48:09 PM
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7th October, 2021
File No. USG1276

Eagle Builders LP
27312 - 17 Twp. Rd. 394
Aspelund Industrial Park
Blackfalds, Alberta
T0M 0J0

ATTENTION: Mr. Allan Prediger, Project Manager

Dear Mr. Prediger,

SUBJECT: Double T Cattle Co.
Proposed Confined Feedlot
N.W. ¼ of 15-42-25 W4M
Ponoka County, Alberta

1 INTRODUCTION

Union Street Geotechnical Ltd. (Union Street) was retained by Eagle Builders LP (Eagle Builders) to perform a field investigation, and subsequent laboratory testing on the subgrade, to aid in the design and construction of a proposed confined feedlot for Double T Cattle Co. located within the N.W. ¼ of 15-42-25 W4 in Ponoka County, Alberta, as shown on Drawing No. A1. Nine boreholes were advanced in the proposed pen and catch basin development footprint for liner design purposes. Based on the boreholes advanced, it was determined that the upper subgrade is predominantly composed of till overlying mudstone.

2 DESCRIPTION OF THE PROJECT AND SITE

2.1 SITE DESCRIPTION

The site is located approximately 1.5 km north-northwest of the intersection of Range Road 252 and Township Road 422, within the N.W. ¼ of 15-42-25 W4 in

4726 - 78A Street Close
Red Deer, Alberta
T4P 2J2

Bus: 403-350-9688
www.unionstreetgeo.ca

Ponoka County, Alberta, as shown on Drawing No. A1. The proposed feedlot development site within the N.W. ¼ was relatively flat with the geological drainage of the area sloping east.

The proposed feedlot is currently utilized as agricultural land and bordered by agricultural land to the north, east, south, with the existing agricultural development to the west-southwest of the proposed feedlot. Photographs depicting the site are attached to this report.

2.2 PROPOSED DEVELOPMENT

The proposed development consists of feedlot pens, catch basin, and associated development. Specific development details are unknown at the time of this report writing but are assumed to be typical to those in the area and for developments of this nature.

Recommendations contained in this report have been given for the above described development and those typical of a development of this nature. If there are any changes to the proposed development, or their locations, these changes should be reviewed by Union Street personnel to confirm the applicability of this report to the revised development plans.

3 FIELD INVESTIGATION AND LABORATORY ANALYSIS

The field investigation program included drilling nine boreholes at the locations shown on Drawing No. A2. The borehole locations were established by Union Street personnel based on a discussion with the owner, proposed development footprint, utility clearance, and access. No formal surveying of the borehole locations or site were completed and therefore, all drawings, locations, measurements, and legal descriptions are approximate and conceptual in nature.

On 14th September, 2021, nine boreholes (designated as BH101 to BH109) were advanced using a truck-mounted auger drill utilizing 150 mm diameter, continuous flight augers, operated by Venom Environmental Drilling Ltd. The boreholes were advanced to a depth of 4.57 m to 9.14 m below ground surface.



3.1 GENERAL STRATIGRAPHY

The subsurface conditions were relatively uniform in all nine borehole locations for liner design purposes. In general, and to the depths drilled, the soil conditions encountered at the borehole locations generally consisted of, in descending order; topsoil overlying till and mudstone. Topsoil, with an approximate average thickness of 320 mm, was encountered at surface in all nine boreholes. Till, with varying clay, silt, and sand content was encountered underlying the topsoil in all nine boreholes which extended to an average depth of 3.13 m below grade. It was yellowish brown (10YR 5/8) to dark grey (10YR 4/1), oxidized to non-oxidized, moist, soft to hard, massive, contained gravel, silt pocket, and coal chip inclusions, and was calcareous. A thin sand seam was encountered within the till in Borehole BH106. Mudstone was encountered underlying the till in all nine boreholes which extended to the maximum exploration depth. The mudstone generally consisted of clayey silt with trace to some sand. Observations made during the field investigation, visual descriptions of the soils, and the results of laboratory analysis are presented in the attached Borehole Logs and Laboratory Test Results.

Undisturbed samples were collected within the till encountered in Borehole BH102 to aid in the proposed catch basin liner design and within the till encountered in Borehole BH109 to aid in the proposed pen liner design. Sample MW16, obtained from Borehole BH102 at 4.57 m below grade, was submitted for hydraulic conductivity testing which indicated a permeability value of 7.87×10^{-9} cm/s. Sample MW48, obtained from Borehole BH109 at 3.05 m below grade, was submitted for hydraulic conductivity testing which indicated a permeability value of 6.61×10^{-7} cm/s.

Three MUSC tests were performed on till samples obtained from Boreholes BH101, BH105, and BH107. The MUSC results are summarized in Table 3.1.



TABLE 3.1: SUMMARY OF TILL MODIFIED UNIFIED SOILS CLASSIFICATION TEST RESULTS

Sample No. and Depth	Borehole No.	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Moisture Content (%)	MUSC – Soil Type
MW3 - 2.29 m	BH101	51.9	11.8	40.1	16.5	CH
MW29 - 0.76 m	BH105	52.4	11.3	41.1	11.4	CH
MW35 - 0.76 m	BH107	40.1	13.0	27.1	12.2	CI
Average:		48.1	12.0	36.1	13.4	CI-CH

Based on the results in Table 3.1 the till has an average MUSC of “CI-CH” - Silts or Clays of medium to high plasticity.

Four Mechanical Wash Sieves (MWSs) were performed on till samples obtained from Borehole BH101, BH105, BH107, and BH109. The MWS sieve results are summarized in Table 3.2.

TABLE 3.2: SUMMARY OF TILL MUSC AND PSA TEST RESULTS

Sample No. and Depth	Borehole No.	Gravel Content (%)	Sand Content (%)	Silt Content (%)	Clay Content (%)
MW3 - 2.29 m	BH101	0.4	33.2	66.4	
MW29 - 0.76 m	BH105	1.9	31.5	66.6	
MW35 - 0.76 m	BH107	0.6	33.5	65.8	
MW45 - 0.76 m	BH109	0.1	67.5	32.4	
Average:		0.8	41.4	57.8	

Cobbles and boulders were not encountered during drilling, but as till is a heterogeneous mixture of all grain sizes, cobbles and boulders may be encountered during construction.

A Particle Size Analysis (PSA) and MWS were performed on mudstone samples obtained from Borehole BH101. The PSA and MWS results are summarized in Table 3.3.



TABLE 3.3: SUMMARY OF MUDSTONE SIEVE AND HYDROMETER TEST RESULTS

Sample No. and Depth	Borehole No.	Gravel Content (%)	Sand Content (%)	Silt Content (%)	Clay Content (%)
MW5 - 3.81 m	BH101	0.0	17.7	82.3	
MW7 - 5.33 m	BH101	0.0	0.9	67.9	31.2
Average:		0.0	9.3	54.5	36.2

Notes:

1 - The fines results of MW5 were split 50/50 in the average.

3.2 GROUNDWATER

Seepage was observed in Boreholes BH101 to BH103 at an average depth of 6.71 m below grade and three piezometers were installed following drilling. The groundwater elevation was recorded on the 28th September, 2021, 14 days following drilling, and the results are summarized in Table 3.4.

TABLE 3.4: SUMMARY OF GROUNDWATER MEASUREMENTS

Borehole No.	Borehole Depth (m)	Groundwater Level ¹ (m), 28 th September, 2021
BH101	7.93	2.47
BH108	9.14	2.68
BH109	4.57	2.63
Average:		2.59

Notes:

1 - Below existing grade.

Based on seepage encountered during drilling and the groundwater elevations recorded in the piezometers, the average groundwater table is likely (approx.) 2.0 m to 3.0 m below ground surface. Groundwater levels are subject to meteorological events, seasonal variations, site gradient, and other salient factors resulting in the water table varying with time.



4 REFERENCES

The following was referenced while composing this letter:

- Province of Alberta, “*Agricultural Operation Practices Act and Regulations*”, Revised Statutes of Alberta 2000, Chapter A-7, Alberta Queen’s Printer, 2010;
- Province of Alberta, “*Agricultural Operation Practices Act and Regulations*”, Standards and Administration Regulation, Part 2, Alberta Queen’s Printer, 2017;
- Alberta Government, “*Catch Basin Design and Management*”, Technical Guideline Agdex 096-101, August 2012;
- Natural Resources Conservation Board, “*Determining Equivalent Protective Layers and Constructed Liners*”, Technical Guideline Agdex 096-61, May 2013; and,
- AL-Terra Engineering (Red Deer) Ltd., “*Site Location & Runoff Control Catch Basin Plan*”, Project No. 5457, dated 6th October, 2021.

5 FEEDLOT PENS

5.1 STRIPPING

All organic soil, vegetation, sand, etc. should be stripped from the feedlot footprint prior to the start of feedlot grading construction activities.

5.2 NATURALLY OCCURRING LINER

The Natural Resources Conservation Board (NRCB) requires naturally occurring protective layers for solid manure collection and storage facilities, such as feedlots, to have a minimum thickness of 2.0 m and a maximum hydraulic conductivity of 1.0×10^{-6} cm/s. Based on the average thickness of the till stratum encountered in the six boreholes advanced in/near the proposed feedlot footprint, 2.79 m, and the



factored hydraulic conductivity test result of the till in Borehole BH109, 6.61×10^{-6} cm/s, the encountered native till in the vicinity of Borehole BH109 does not meet the naturally occurring protective layer requirement.

5.3 COMPACTED SOIL LINER

The NRCB requires compacted soil liners for solid manure collection and storage facilities, such as feedlot pens, to have a minimum thickness of 0.5 m and a maximum hydraulic conductivity of 5.0×10^{-7} cm/s. A hydraulic conductivity analysis was performed on a native till sample obtained from Borehole BH102 at 4.57 m below grade and which had a factored result of 7.87×10^{-8} cm/s. Native till utilized as fill obtained from the vicinity of Borehole BH102 (eastern portion of the site) compacted to a minimum $1,902 \text{ kg/m}^3$ dry density at 15.1% moisture will be suitable as a clay liner across the feedlot and meets NRCBs soil liner requirement.

If a compacted soil liner is utilized, the NRCB requires the bottom of the soil liner to be equal or greater than 1.0 m from the groundwater table at the time of construction.

5.4 CONCRETE LINER

The client has indicated that the pens will likely utilize a concrete liner. If concrete is utilized, it must offer the equivalent protection of a 0.50 m thick soil liner with a permeability of not more than 5.0×10^{-7} cm/s. The type of concrete proposed for the liner is unknown at this time, but is expected to well exceed this requirement.

5.5 GRADING

The base of the pens must be positively graded, to ensure liquids don't pond on the subgrade, to a catch basin or other runoff control system. It is assumed that till, from cut/fill grading activities, will be utilized during construction if fill is required. Due to its plasticity, the till isn't recommended for areas requiring structural fill.

Fill, composed of native till, should be placed in lifts not exceeding 200 mm and compacted to a minimum 98% of its Standard Proctor Dry Density (SPDD) at moisture contents +2% of optimum. The local soils may require moisture



conditioning to achieve the required degrees of compaction. The degree to which moisture conditioning of the fill would be required may vary with the local soils and construction season. There may also be some localized areas where the native soils may require drying, or blending with drier soils, in order to achieve the required degrees of compaction.

6 CATCH BASIN

6.1 CAPACITY

For preliminary design purposes, the design volume of the catch basin must have a storage capacity that can accommodate a 1 in 30 year rainfall. For the Ponoka region a 1 in 30 year event equates to approximately 80 mm of rainfall. The drainage area of the feedlot, including the proposed catch basin, is approximately 79,703 m². The following was utilized to determine the catch basins minimum required capacity.

$$V_{30} = D_A \times R_{30} \times C_R$$

Where:

V_{30} = One Day Rainfall Volume (m³);

D_A = Drainage Area (m²);

R_{30} = One Day Rainfall (m); and,

C_R = Runoff Coefficient (1.0 for a paved area).

Based on the referenced formula, it has been determined that the expected one day rainfall volume for the site is approximately 6,376.2 m³. However, to ensure the liners integrity due to drying out and cracking, and to increase the timeframe between emptying, the design capacity of the catch basin should be greater than the 1 in 30 year rainfall volume. Union Street recommends increasing the total volume capacity by approximately 60% of the 1 in 30 year rainfall minimum volume to approximately 10,137 m³.

The size and capacity of the catch basin may change depending on the liner option selected as, for example, a synthetic liner will allow a deeper catch basin, allowing a reduced footprint, reducing the required capacity. Therefore, although the general



footprint will remain similar, the size and location of the catch basin shown on the attached drawing may slightly differ from that actually constructed.

The catch basin must have a marker that is clearly visible at all times indicating the minimum volume required to accommodate a 1 in 30 year one day rainfall event.

6.2 STRIPPING

All organic soil and vegetation should be stripped from the catch basin footprint prior to the start of catch basin construction activities.

6.3 CATCH BASIN EXCAVATION

All till material from the catch basin excavation that is determined to be suitable for reuse should be stockpiled.

The banks of the catch basin should be cut at no steeper than 3H:1V. The capacity of the catch basin should be designed ensuring a minimum 0.5 m freeboard. It is the responsibility of the contractor to remove water from trenches and excavations, regardless of origin. If while constructing the slopes of the catch basin subsurface, groundwater begins eroding the slopes and entering the catch basin, construction will need to be halted immediately and dewatering techniques will need to be implemented before construction continues. It is anticipated that potential groundwater problems can be resolved with well graded ditching and the installation of subgrade sumps around the perimeter of the site. If extreme groundwater seepage becomes present, more advanced dewatering techniques can be implemented. Although possible, it is not expected that seepage and sloughing will be encountered during construction unless excavations exceed 2.6 m in depth.

Pumps and other materials necessary to keep the excavation free of water while work is in progress should be provided. Provisions should be made in case of accidental stoppage of dewatering equipment to prevent damage to the work area. The excavations must be protected against flooding and damage from surface run-off. Water removed from the site is to be disposed of in a manner that will not damage the work area or other property or persons.



Materials will be excavated and removed to the depths necessary for the construction of the structure and drainage system. Care must be taken to minimize the disturbance to the supporting soil. After the excavation has been shaped, any over-excavated areas will be backfilled and compacted to a density equal to or greater than the undisturbed soil. All slopes in the subgrade are to be uniform and in a condition suitable for a catch basin.

6.4 EMBANKMENTS AND FILL

An embankment/berm is recommended to be constructed around the perimeter of the feedlot to divert and minimize surface runoff from outside the operation from flowing into the catch basin. Additionally, a berm is recommended along the perimeter of the catch basin to prevent accidental effluent release outside of the operation and ensure a minimum 0.5 m freeboard. The exterior slope of a catch basin wall should be no steeper than 4H:1V. Any fills required can be constructed from the till subgrade encountered on-site. If an insufficient quantity of suitable on-site subgrade fill is not available, it will have to be analysed, imported, and compacted.

Areas requiring fills will be uniformly graded, scarified and re-compacted to the necessary density prior to being filled. Common excavated materials will be placed in the embankments, and in over-excavations if approved by the Geotechnical Engineer. Fills should be placed in lifts not exceeding 200 mm and compacted to minimum 98% of the SPDD at +2% Moisture. Fill material may require moisture conditioning prior to compaction.

6.5 LINER

6.5.1 Naturally Occurring Soil Liner

Following a review of the referenced NRCB documentation, it is understood that a naturally occurring protective layer for a catch basin must have a minimum thickness of 5.0 m and a maximum hydraulic conductivity of 1.0×10^{-6} cm/s. Additionally, the groundwater table must be approximately 1.0 m below the bottom of the naturally occurring liner. Laboratory testing was conducted on an undisturbed till sample in Borehole BH101 with a hydraulic conductivity of 7.87×10^{-9} cm/s. However, NRCB

requires laboratory permeability results to be reduced by an order of magnitude. When reduced by this magnitude, the design hydraulic conductivity of the till in the catch basin location is assumed to be 7.87×10^{-8} cm/s. Based on the average thickness of the till stratum encountered in Boreholes BH101 to BH103 and BH108 (boreholes advanced in/near the proposed catch basin footprint) and the factored hydraulic conductivity of the till, 2.80 m and 7.87×10^{-8} cm/s respectively. As such, a naturally occurring till layer 0.39 m thick with a permeability of 7.87×10^{-8} cm/s offers the equivalent protection of a 5.0 m thick layer with a permeability of 1.0×10^{-6} cm/s. A minimum liner thickness of 0.5 m is recommended however to ensure its structural integrity.

If a naturally occurring soil liner is utilized, the NRCB requires the bottom of the soil liner to be equal or greater than 1.0 m from the groundwater table at the time of construction.

6.5.2 Compacted Soil Liner

A compacted soil liner could be utilized at this site, but as the till's naturally occurring permeability meets the referenced requirements, it is unlikely that a compacted soil liner would be utilized at this site.

6.5.3 Geomembrane

If a catch basin with a great depth is required, to reduce the overall footprint, a synthetic liner can be used. If utilized, all geomembrane products should be handled, stored, and placed in accordance with the manufacturer's recommendations. Materials should be stored so that they do not come into contact with substances that may affect their physical or chemical properties such as fuel, exhausts, or petroleum products.

The installation contractor should be a contractor approved by the civil engineer who is trained to install the manufacturer's geomembrane. Installation should be performed by personnel experienced in seaming the materials under the constant supervision of the manufacturer. It is recommended that the installation contractor provide a written report on the completed installation certifying that the liner was



installed in accordance with the requirements of the manufacturer's specifications, the liner is ready for operation, and the warranty is in effect.

Geotextiles should be sufficiently anchored and deployed in a manner that will reduce folds and wrinkles. In the presence of wind, geotextiles should be weighted with sandbags or equivalent ballast. Geotextiles are to be cut using an approved cutter. Care should be taken in the installation process not to entrap excessive dust or stones that could damage the geomembrane.

The contractor should submit a panel layout proposal for the geomembrane to the engineer prior to the geomembrane placement. Care should be taken in the method used to unroll the panels so that damage to the liner or the supporting soil and/or geomembrane. Sand bags or equivalent ballast that will not damage the liner should be placed on the liner to prevent uplift. No equipment or tools that could damage the liner or underlying surfaces by handling or other means should be used. No personnel working on the liner will wear damaging shoes or engage in activities that could harm the liner, including smoking. All defects and damage will be documented and marked for repair. Repairs will be conducted in a manner suitable to the geomembrane manufacturer.

No NRCB technical specifications regarding synthetic liner were found. If utilized, it is recommended to discuss the liner requirements with the manufacturer and once a product has been selected, to discuss the technical specifications with the NRCB.

Pumping may be required during liner placement if the excavation starts to fill with groundwater. It is recommended that the means be available to prevent "bubbling" of the liner if groundwater starts to form below the liner in the catch basin footprint.

Seepage and sloughing may be encountered in the till subgrade depending upon the base elevation of the catch basin.

If a synthetic liner is utilized, the NRCB requires the bottom of the liner to be equal or greater than 1.0 m from the groundwater table at the time of construction.



6.6 QUALITY CONTROL / QUALITY ASSURANCE

As part of the quality control program, it is recommended that a geotechnical engineer or representative be on-site to inspect the excavation and compaction required. The geotechnical engineer will be able to provide immediate on-site recommendations to potential difficulties that may arise during construction.

6.7 INLET PIPE

It is understood multiple inlet pipes will be utilized for the construction of the catch basin. The inlet pipes must be sealed to ensure liquid manure doesn't seep back along the pipe extrusion, creating a potential source of contamination. Bentonite chips or concrete are typically utilized around the inlet pipe to create the required seal.

6.8 EROSION

Due to the catch basin's size or liner type, these measures may not be necessary, but unchecked erosion can lead to slope and berm failure and erosion preventative measures may be required. Placing riprap is normally the most cost effective erosion protection material, placed on the waterward side, due to its effectiveness, durability and availability.

Additionally, exposed soil should be graded to the required slope, overlain with topsoil, and seeded or hydroseeded with grass. Trees and shrubs planting is not recommended as tree roots detrimentally affect berms by root penetration and shrubs cause obstructions in viewing piping, seepage, and burrowing animals. The vegetation will serve to protect the upper portions of the slope from erosion by surface runoff water and will also increase the stability of the slope. The grass should be trimmed regularly as to not obstruct the inspectors view.

6.9 FENCE

It is recommended that continuous fencing around the perimeter of the catch basin is constructed. A fence will help prevent unauthorized entry to the catch basin and will also help reduce the detrimental effects of burrowing animals such as beavers, muskrats, gophers, etc.



6.10 INSPECTIONS

It is the responsibility of the owner to conduct routine and periodic inspections and to maintain and repair the catch basin to acceptable standards. It is recommended that the catch basin is inspected on a regular basis or as per the Natural Resources Conservation Board. The inspector shall note, but not be limited to noting, the presence or absence of settlement, seepage, burrowing animals, erosion, freeboard level, erosion protection performance and condition, fence condition, vegetation growth that may lead to a decreased performance of the liner, and general berm and catch basin condition.

7 CLOSURE

Union Street Geotechnical Ltd. prepared this report for the exclusive use of Eagle Builders LP, and their agents, to aid in the design and construction of the proposed Double T Cattle Co. feedlot located within the N.W. $\frac{1}{4}$ of 15-42-25 W4 in Ponoka County, Alberta. The content reflect Union Street's best judgement available to it at the time of preparation. Any use which a third party makes of this report, or any reliance on, or decisions to be made based on it, are the responsibility of such third party and Union Street accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

Our recommendations and conclusions are based upon the information obtained from the subsurface exploration. The borings and associated laboratory testing indicate subsurface conditions only at the time and to the depth, of the specific boring location investigated and only for the soil properties tested. The subsurface conditions may vary between the boreholes and over time. The interpretation of subsurface conditions provided is a professional opinion of encountered conditions and is not a certification or guarantee of site conditions. If variations, or other latent conditions become evident, Union Street should be notified immediately so that our conclusions and recommendations can be re-evaluated. Although subsurface conditions have been explored, we have not conducted investigations, sampling, field or laboratory testing, evaluations, or modelling of the site or subsurface conditions with respect to the presence of contaminated soil or groundwater or slope stability conditions.



This report contains the results of our geotechnical investigation as well as certain recommendations arising from our investigation. The general recommendations herein do not constitute a design, in whole or in part, of any of the structural elements of the proposed work. Incorporation of any or all of our general recommendations into the design of any such element does not constitute us as designers or co-designers of such elements, nor does it mean that such design is appropriate in geotechnical terms. The designers of such elements must consider the appropriateness of our general recommendations in light of all design criteria known to them, many of which are not known by us. Our mandate has been to perform a geotechnical investigation and provide general site suitability recommendations, which we have completed by means of this report. We have had no mandate to design, or review the design of any elements of the proposed work and accept no responsibility for such design or design review.

This report has been prepared in accordance with generally accepted geotechnical engineering practice common to the local area. No other warranty, expressed or implied, is made.

This document, and the information contained within, are the confidential property of Eagle Builders LP and any disclosure of same is governed by the provisions of each of the applicable provincial or territorial Freedom of Information legislation, the Privacy Act (Canada) 1980-81-82-83, c.111, Sch. II "2", and the Access to Information Act (Canada) 1980-81-82-83, c.111, Sch. I "1", as such legislation may be amended or replaced from time to time.



Yours truly,

Union Street Geotechnical Ltd.

Prepared By:



Neil Tomaszewski, E.I.T.
Project Engineer

Reviewed By:



Joshua Wilson, P.Eng.
Geotechnical Manager

Union Street Geotechnical Ltd.

APEGA Permit No. P12644



7th October, 2021
APEGA ID# 80317



ATTACHMENTS

DRAWINGS

Drawing No. A1 - Site Location Plan

Drawing No. A2 - Borehole Location Plan

PHOTOGRAPHS

Photographs No. 1 & 2

BOREHOLE LOGS

Boreholes No. BH101 to BH109, inclusive

LABORATORY TEST RESULTS

Flexible Wall Permeameter - Sample No. MW16

Flexible Wall Permeameter - Sample No. MW48

Laboratory Hydrometer - Sample No. MW7





Drawings



LEGEND	
	SITE BOUNDARY
	ROAD RIGHT OF WAY
	PROPERTY LINE (WHITE)

GENERAL NOTES

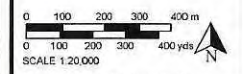
1. DRAWINGS COMPILED FROM
 MULTIPLE VECTOR AND RASTER GIS
 FIELDS AS PUBLISHED BY THE
 GOVERNMENT OF CANADA, THE
 GOVERNMENT OF ALBERTA,
 NATURAL RESOURCES CANADA,
 EARTH SCIENCES SECTOR, CANADA
 CENTRE FOR MAPPING AND EARTH
 OBSERVATION, HAMLET OF NISQU.

AREIAL PHOTOGRAPH (C) 2020
 MAXAR TECHNOLOGIES (C) GOOGLE
 EARTH PRO.

UNION STREET GEOTECHNICAL
 INVESTIGATION.

PDF SKETCH PROVIDED BY THE
 CLIENT

2. LEGAL DESCRIPTION:
 A PORTION OF N.E. 1/4 15-42-25-W4M



NOTE: SCALE IS FOR REFERENCE
 PURPOSES ONLY AND IS APPROXIMATE IN
 NATURE.

No.	REVISION/ISSUE	DATE

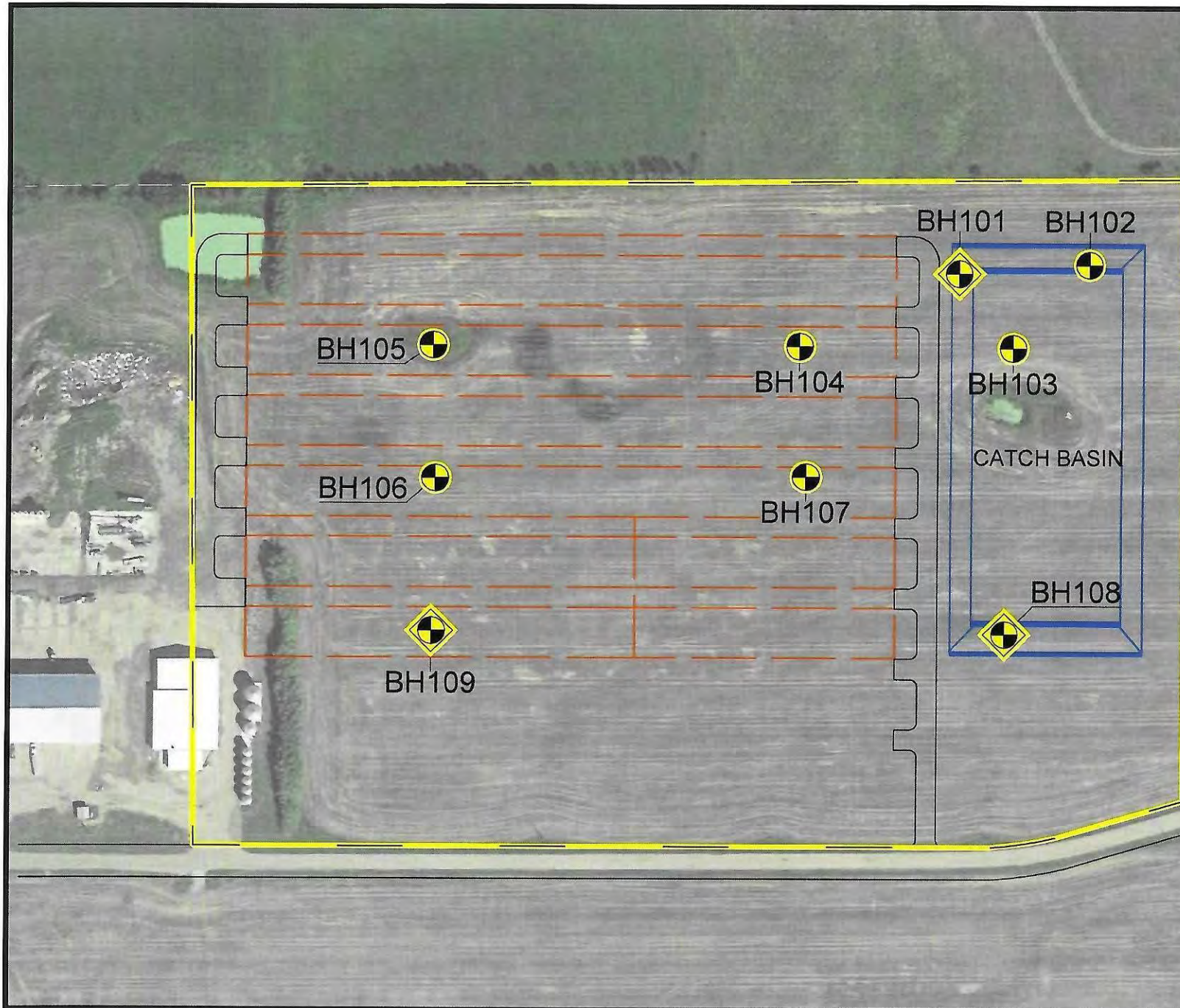


PROJECT NAME AND ADDRESS
 GEOTECHNICAL INVESTIGATION
 DOUBLE T CATTLE CO.
 N.E. 1/4 OF 15-42-25-W4M
 PONOKA COUNTY,
 ALBERTA

PROJECT GEOTECHNICAL INVESTIGATION	SHEET
PROJECT NUMBER USG1276	A1
DATE 24/09/2021	
DRAWING NAME SITE LOCATION PLAN	

USG1276 - Drawing.dwg

30/09/2020



LEGEND

- ROADWAY
- PROPERTY LINE (WHITE)
- SITE BOUNDARY
- PROPOSED BUILDINGS
- PROPOSED CATCH BASIN
- BOREHOLE LOCATION
- PIEZOMETER LOCATION

GENERAL NOTES

- DRAWINGS COMPILED FROM
 - MULTIPLE VECTOR AND RASTER GIS FILES AS PUBLISHED BY THE GOVERNMENT OF CANADA, THE GOVERNMENT OF ALBERTA, NATURAL RESOURCES CANADA, EARTH SCIENCES SECTOR, CANADA CENTRE FOR MAPPING AND EARTH OBSERVATION, CITY OF GRANDE PRAIRIE.
 - AREAL PHOTOGRAPH (C) 2020 MAXAR TECHNOLOGIES (C) GOOGLE EARTH PRO.
 - UNION STREET GEOTECHNICAL INVESTIGATION,
- & PDF SKETCH PROVIDED BY THE CLIENT
- LEGAL DESCRIPTION
 - A PORTION OF N.E. 14-15-42-25-W4M

0 10 20 30 40 50 m
0 50 100 150 ft
SCALE 1:2,500

NOTE SCALE IS FOR REFERENCE PURPOSES ONLY AND IS APPROXIMATE IN NATURE.

No	REVISION/ISSUE	DATE

UNION STREET GEOTECHNICAL LTD.
Union Street Geotechnical

PROJECT NAME AND ADDRESS
GEOTECHNICAL INVESTIGATION
DOUBLE T CATTLE CO.
N.E. 1/4 OF 15-42-25-W4M
PONOKA COUNTY,
ALBERTA

PROJECT GEOTECHNICAL INVESTIGATION	SHEET
PROJECT NUMBER USG1276	A2
DATE 24.09.2021	
DRAWING NAME BOREHOLE LOCATION PLAN	



Photographs

**Photographs - Geotechnical Investigation
N.W. ¼ of 15-42-25 W4
Ponoka County, Alberta**



Photograph No. 1: Photograph taken from near the western edge of the proposed feedlot, facing east, showing the proposed feedlot footprint, site grading, agricultural land, and general site conditions observed at the time of drilling. Photograph taken on the 14th September, 2021.



Photograph No. 2: Photograph taken from near the eastern edge of the proposed feedlot, facing west, showing the proposed feedlot footprint, site grading, agricultural development to the west-southwest, and general site conditions observed at the time of drilling. Photograph taken on the 14th September, 2021.



Borehole Logs

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH101

PROJECT NUMBER: USG1276
 PROJECT NAME: Geotechnical Investigation
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: Eagle Builders LP
 DRILLING METHOD: 150 mm Solid Stem Auger
 LOGGED BY: M.W.
 DATE BEGUN: 14 September, 2021
 DATE COMPLETED: 14 September, 2021

CASING STICKUP: 1.00 m
 TOTAL DEPTH: 7.93 m
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION		
			TYPE	No.	SPT "N"									
-1.0														
0.0		TOPSOIL: 356 mm thick.										Cap.		
1.0		TILL: Clay, silty, sandy. Very dark brown (10YR 2/2). Oxidized. Moist. Stiff to very stiff. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW1		192	11.8					Solid 25 mm PVC casing.		
2.0				MW2	12	132	16.0							
2.5				MW3		96	16.5		CH	51.9	11.8			
3.5				MW4	19	96	19.7							
4.0			MUDSTONE: Silty, clayey, some sand. Dark grey (10YR 4/1). Non-oxidized. Dry to moist. Very stiff to hard. Massive. Calcareous.		MW5		144	16.0		CH	55.0		19.5	Auger cuttings.
5.0					MW6	Ref.	215	15.4						
5.5					MW7		-	15.1						
6.5					MW8	Ref.	144	15.1						
7.0					MW9		-	20.9						
8.0					MW10	Ref.	96	17.6						
8.5				MW11		48	23.3							
9.0													Hand slotted 25 mm PVC.	

NOTES: End of borehole at 7.93 m below surface. Seepage, but no sloughing encountered during drilling. Piezometer installed, annulus backfilled to surface with auger cuttings. Water level at 2.47 m below grade on 28 September, 2021.

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH102

PROJECT NUMBER: **USG1276**
 PROJECT NAME: **Geotechnical Investigation**
 LOCATION: **N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta**
 CLIENT: **Eagle Builders LP**
 DRILLING METHOD: **150 mm Solid Stem Auger**
 LOGGED BY: **M.W.**
 DATE BEGUN: **14 September, 2021**
 DATE COMPLETED: **14 September, 2021**

CASING STICKUP: **N/A**
 TOTAL DEPTH: **9.14 m**
 GROUND SURFACE ELEVATION: **N/A**



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 356 mm thick.										
1.0		TILL: Clay, silty, some sand. Dark yellowish brown (10YR 4/4) to dark brown (10YR 3/3). Oxidized. Moist. Very stiff. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW12		192	10.2					
2.0				MW14		144	15.7					
4.0		@ 4.88 m, seepage.		MW15		-	15.7					
5.0		MUDSTONE: Silt, clayey, some to trace sand. Dark grey (10YR 4/1). Non-oxidized. Dry to moist. Very stiff to hard. Massive. Calcareous.		MW16		-	16.1					
6.0				MW17		-	26.1					
7.0				MW18		-	27.9					
8.0				MW19		-	29.2					
9.0		NOTES: End of borehole at 9.14 m below surface. Seepage and sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
10.0												

Auger cuttings.

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH103

PROJECT NUMBER: USG1276
 PROJECT NAME: **Geotechnical Investigation**
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: **Eagle Builders LP**
 DRILLING METHOD: **150 mm Solid Stem Auger**
 LOGGED BY: **M.W.**
 DATE BEGUN: **14 September, 2021**
 DATE COMPLETED: **14 September, 2021**

CASING STICKUP: N/A
 TOTAL DEPTH: **9.14 m**
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 305 mm thick.										
1.0		TILL: Clay, silty, some sand. Yellowish brown (10YR 5/8) to dark grey (10YR 4/1). Oxidized. Moist. Hard. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW20		215	12.0					
2.0				MW21		215	17.8					
3.0		MUDSTONE: Silt, clayey, some to trace sand. Grey (10R 5/1) to dark grey (10YR 4/1). Non-oxidized. Dry to moist. Very stiff to hard. Massive. Calcareous.		MW22		-	14.8					
4.0				MW23		-	15.7					
5.0				MW24		-	28.8					
6.0				MW25		-	29.9					
7.0		@ 7.62 m, seepage.										
8.0												
9.0		NOTES: End of borehole at 9.14 m below surface. Seepage and sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
10.0												

Auger cuttings.

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH104

PROJECT NUMBER: USG1276
 PROJECT NAME: Geotechnical Investigation
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: Eagle Builders LP
 DRILLING METHOD: 150 mm Solid Stem Auger
 LOGGED BY: M.W.
 DATE BEGUN: 14 September, 2021
 DATE COMPLETED: 14 September, 2021

CASING STICKUP: N/A
 TOTAL DEPTH: 4.57 m
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 356 mm thick.										
1.0		TILL: Clay, silty, some sand. Yellowish brown (10YR 5/6) to dark greyish brown (10YR 4/2). Oxidized. Moist. Very stiff. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW26		144	9.4					
2.0				MW27		144	15.5					
4.0		MUDSTONE: Silt, clayey, some to trace sand. Pale Brown (10YR 6/3). Non-oxidized. Dry to moist. Very stiff to hard. Massive. Calcareous.		MW28		168	20.2					
5.0		NOTES: End of borehole at 4.57 m below surface. No seepage or sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
6.0												
7.0												
8.0												
9.0												
10.0												

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH105

PROJECT NUMBER: USG1276
 PROJECT NAME: **Geotechnical Investigation**
 LOCATION: **N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta**
 CLIENT: **Eagle Builders LP**
 DRILLING METHOD: **150 mm Solid Stem Auger**
 LOGGED BY: **M.W.**
 DATE BEGUN: **14 September, 2021**
 DATE COMPLETED: **14 September, 2021**

CASING STICKUP: **N/A**
 TOTAL DEPTH: **4.57 m**
 GROUND SURFACE ELEVATION: **N/A**



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 305 mm thick.										
1.0		TILL: Clay, silty, sandy. Brown (10YR 4/3). Oxidized. Moist. Hard. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW29		215	11.4		CH	52.4	11.3	 Auger cuttings.
2.0		MUDSTONE: Silt, clayey, some to trace sand. Dark grey (10YR 4/1). Non-oxidized. Dry to moist. Stiff to hard. Massive. Calcareous.		MW30		96	24.1					
4.0				MW31		-	17.6					
5.0		NOTES: End of borehole at 4.57 m below surface. No seepage or sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
10.0												

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH106

PROJECT NUMBER: USG1276
 PROJECT NAME: Geotechnical Investigation
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: Eagle Builders LP
 DRILLING METHOD: 150 mm Solid Stem Auger
 LOGGED BY: M.W.
 DATE BEGUN: 14 September, 2021
 DATE COMPLETED: 14 September, 2021

CASING STICKUP: N/A
 TOTAL DEPTH: 4.57 m
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 305 mm thick.										
1.0		TILL: Clay, silty, sandy. Yellowish brown (10YR 5/6). Oxidized. Moist. Stiff to very stiff. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW32		96	12.6					Auger cuttings.
2.0		@ 0.76 m, sand seam, 0.30 m thick.		MW33		144	18.0					
4.0		MUDSTONE: Silt, clayey, some to trace sand. Very pale brown (10YR 7/4). Non-oxidized. Dry to moist. Hard. Massive. Calcareous.		MW34		-	16.5					
5.0		NOTES: End of borehole at 4.57 m below surface. No seepage or sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
6.0												
7.0												
8.0												
9.0												
10.0												

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH107

PROJECT NUMBER: USG1276
 PROJECT NAME: **Geotechnical Investigation**
 LOCATION: **N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta**
 CLIENT: **Eagle Builders LP**
 DRILLING METHOD: **150 mm Solid Stem Auger**
 LOGGED BY: **M.W.**
 DATE BEGUN: **14 September, 2021**
 DATE COMPLETED: **14 September, 2021**

CASING STICKUP: **N/A**
 TOTAL DEPTH: **4.57 m**
 GROUND SURFACE ELEVATION: **N/A**



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 305 mm thick.										
1.0		TILL: Clay, silty, sandy. Dark greyish brown (10YR 4/2). Oxidized. Moist. Hard. Massive. Coal chip, gravel, and silt pocket inclusions. Calcareous.		MW35		215	12.2		CI	40.1	13.0	Auger cuttings.
2.0		@ 1.52 m, sand, some clay, some silt.		MW36		48	19.2					
4.0		MUDSTONE: Silt, clayey, some to trace sand. Dark grey (10YR 4/1). Non-oxidized. Moist. Stiff to hard. Massive. Calcareous.		MW37		96	16.6					
5.0		NOTES: End of borehole at 4.57 m below surface. No seepage or sloughing encountered during drilling. Borehole backfilled to surface with auger cuttings.										
10.0												

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH108

PROJECT NUMBER: USG1276
 PROJECT NAME: Geotechnical Investigation
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: Eagle Builders LP
 DRILLING METHOD: 150 mm Solid Stem Auger
 LOGGED BY: M.W.
 DATE BEGUN: 14 September, 2021
 DATE COMPLETED: 14 September, 2021

CASING STICKUP: 0.88 m
 TOTAL DEPTH: 9.14 m
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
0.0		TOPSOIL: 305 mm thick.										
0.0 - 1.0		TILL: Sand, some clay, some silt. Brownish yellow (10YR 6/8). Oxidized. Moist. Loose. Massive. Calcareous.		MW38		-	10.4					
1.0 - 2.0		MUDSTONE: Silt, clayey, some to trace sand. Light yellowish brown (10YR 6/4) to grey (10YR 5/1). Non-oxidized. Moist. Hard. Massive. Calcareous.		MW39		-	4.8					
2.0 - 4.0				MW40		-	12.9					
4.0 - 5.0				MW41		-	14.0					
5.0 - 6.0				MW42		-	16.2					
6.0 - 7.0				MW43		-	14.6					
7.0 - 8.0				MW44		-	12.0					
8.0 - 9.0												
9.0 - 10.0												

NOTES: End of borehole at 9.14 m below surface. No seepage or sloughing encountered during drilling. Piezometer installed, annulus backfilled to surface with auger cuttings. Water level at 2.68 m below grade on 28 September, 2021.

FIELD BOREHOLE LOG

BOREHOLE NUMBER

BH109

PROJECT NUMBER: USG1276
 PROJECT NAME: Geotechnical Investigation
 LOCATION: N.W. 1/4 of 15-42-25 W4, Ponoka County, Alberta
 CLIENT: Eagle Builders LP
 DRILLING METHOD: 150 mm Solid Stem Auger
 LOGGED BY: M.W.
 DATE BEGUN: 14 September, 2021
 DATE COMPLETED: 14 September, 2021

CASING STICKUP: 0.85 m
 TOTAL DEPTH: 4.57 m
 GROUND SURFACE ELEVATION: N/A



DEPTH (m)	LITHOLOGY	DESCRIPTION	SAMPLE			POCKET PEN (kPa)	MOISTURE CONT. (%)	SULPHATE (%)	USC	LIQUID LIMIT (%)	PLASTIC LIMIT (%)	WELL INSTALLATION
			TYPE	No.	SPT "N"							
-1.0 to 0.0		TOPSOIL: 305 mm thick.										
0.0 to 2.0		TILL: Sand, some clay, some silt. Yellowish brown (10YR 5/8). Oxidized. Moist. Compact. Massive. Calcareous.		MW45		-	15.1					
2.0 to 2.2				MW46		-	-					
2.2 to 2.6				MW47		-	15.3					
2.6 to 3.5				MW48		-	24.5					
3.5 to 4.0		MUDSTONE: Clay, silty, trace sand. Dark grey (10YR 4/1). Non-oxidized. Moist. Hard. Massive. Calcareous.		MW49		-	13.4					
4.0 to 4.57		NOTES: End of borehole at 4.57 m below surface. No seepage or sloughing encountered during drilling. Piezometer installed, annulus backfilled to surface with auger cuttings. Water level at 2.63 m below grade on 28 September, 2021.										



Project Name:		Depth:	4.57 m
Project Number:	USG1276	Testing Company:	Union Street Geo.
Client:		Field Technician:	M.W.
Testhole:	BH102	Sample Date:	
Location:		Lab Technician:	B.B.
Sample Number:	MW6	Date Tested:	September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Material and Test Description

Material Description:			
Clay Till - silty, some sand, trace gravel, oxide inclusion, coal, alkalines, dark grey			
Test Type:	Constant Head	Remoulding Details	
Mould Size:	Flexible Wall	Max Dry Density (kg/m ³):	-
Sample Source:	Shelby Tube	Proctor ID:	-
Fluid Used:	Deaired Water	Percent Max (%):	-
Fluid Reservoir:	Burrettes	Target Dry Density (kg/m ³):	-

Initial Sample Characteristics

Water Content		Sample Size					
Wet + Tare (g):	434.2	Trial	1	2	3	4	Average
Dry + Tare (g):	379.1	Diameter (mm):	73.1	72.6	72.6	73.1	72.9
Tare (g):	14.1	Length (mm):	79.4	79.3	79.2	79.4	79.3
Water Content (%):	15.1%	Weight (g)	723.7				

Area (cm ²):	41.7	Specific Gravity (Note 2):	2.72
Volume (cm ³):	330.6	Void Ratio:	42.9%
Wet Density (kg/m ³):	2189	Saturation:	95.6%
Dry Density (kg/m ³):	1902	Porosity:	30.0%

Final Sample Characteristics

Water Content		Sample Size					
Wet + Tare (g):	748.5	Trial	1	2	3	4	Average
Dry + Tare (g):	644.1	Diameter (mm):	73	73.2	73.1	73.7	73.3
Tare (g):	13.1	Length (mm):	79.9	79.8	79.9	79.9	79.9
Water Content (%):	16.5%	Weight (g)	735.6				

Area (cm ²):	42.1	Specific Gravity (Note 1):	2.72
Volume (cm ³):	336.6	Void Ratio:	44.9%
Wet Density (kg/m ³):	2185	Saturation:	100.0%
Dry Density (kg/m ³):	1875	Porosity:	31.0%

Note 1: Specific gravity for final sample characteristics calculation adjusted to result in 100.0% saturation.

Note 2: Specific gravity for initial sample characteristics calculation set equal to that of the final.

Project Name: _____
 Project Number: USG1276 _____
 Client: _____
 Testhole: BH102 _____
 Location: _____
 Sample Number: MW16 _____

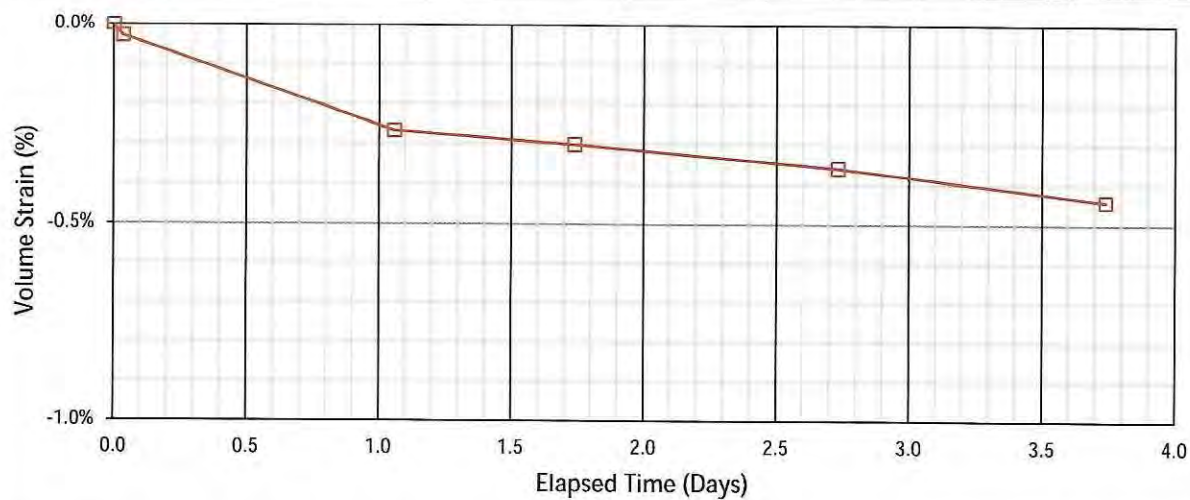
Depth: 4.57 m _____
 Testing Company: Union Street Geo. _____
 Field Technician: M.W. _____
 Sample Date: _____
 Lab Technician: B.B. _____
 Date Tested: September 18, 2021 _____

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Saturation Data

Cell Pressure (kPa):		160.0		Top Pressure (kPa):		130.0	
Bottom Pressure (kPa):		130.0		Pressure Difference (kPa):		-	
Date & Time	Elapsed Time (Days)	Room Temp (°C)	Top Burret (mL)	Bottom Burret (mL)	Cell (mL)	Total Vol. Change (mL)	Volume Strain (%)
9/18/21 14:09	0.00	21.0	3.1	3.2	11.9	0	0.00%
9/18/21 14:59	0.03	21.0	3.2	3.2	11.9	-0.10	-0.03%
9/19/21 15:34	1.06	21.0	3.8	4.1	11.2	-0.89	-0.27%
9/20/21 7:54	1.74	21.0	4.0	4.3	10.9	-1.00	-0.30%
9/21/21 7:47	2.73	21.0	4.1	4.5	10.8	-1.19	-0.36%
9/22/21 7:55	3.74	21.0	4.1	4.6	10.9	-1.46	-0.44%
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-



Project Name: _____
 Project Number: USG1276
 Client: _____
 Testhole: BH102
 Location: _____
 Sample Number: MW16

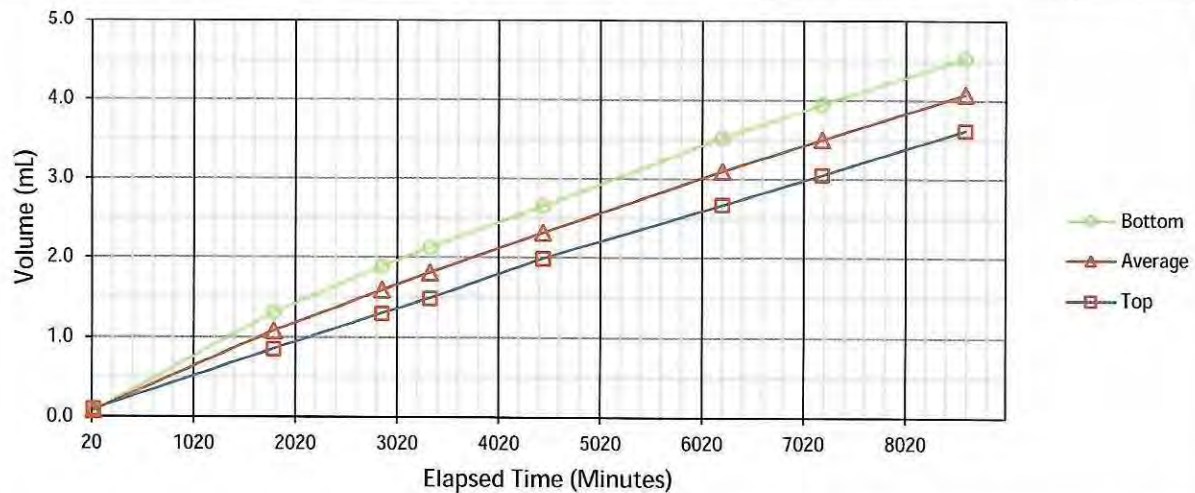
Depth: 4.57 m
 Testing Company: Union Street Geo.
 Field Technician: M.W.
 Sample Date: _____
 Lab Technician: B.B.
 Date Tested: September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Permeation Data

Cell Pressure (kPa):		160.0		Top Pressure (kPa):		120.0	
Bottom Pressure (kPa):		140.0		Pressure Difference (kPa):		20.0	
Date & Time	Elapsed Time (Minutes)	Room Temp (°C)	Top Buret (mL)	Bottom Buret (mL)	Top Vol. Change (mL)	Bottom Vol. Change (mL)	Average Vol. Change (mL)
9/22/21 8:02	0	21.0	9.78	0.11	0.00	0.00	0.00
9/22/21 8:39	37	21.0	9.68	0.18	0.10	0.07	0.08
9/23/21 14:07	1805	21.0	8.93	1.42	0.85	1.31	1.08
9/24/21 7:54	2872	21.0	8.48	1.99	1.30	1.88	1.59
9/24/21 15:44	3342	21.0	8.29	2.24	1.49	2.13	1.81
9/25/21 10:20	4458	21.0	7.79	2.76	1.99	2.65	2.32
9/26/21 15:44	6222	21.0	7.11	3.63	2.67	3.52	3.10
9/27/21 8:05	7203	21.0	6.73	4.06	3.05	3.95	3.50
9/28/21 7:28	8606	21.0	6.16	4.64	3.62	4.53	4.08
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-



Project Name: _____
 Project Number: USG1276
 Client: _____
 Testhole: BH102
 Location: _____
 Sample Number: MW16

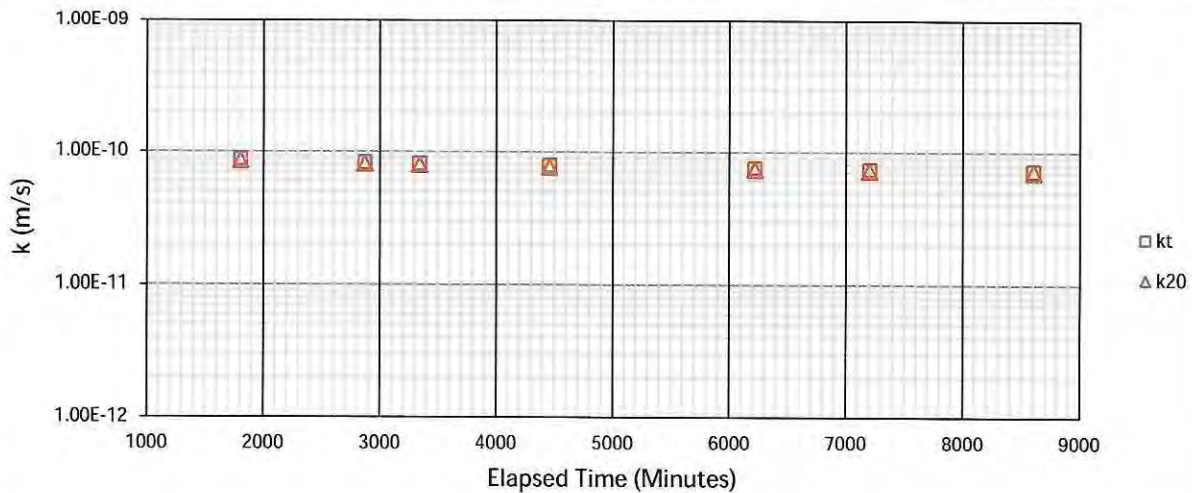
Depth: 4.57 m
 Testing Company: Union Street Geo.
 Field Technician: M.W.
 Sample Date: _____
 Lab Technician: B.B.
 Date Tested: September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Permeation Data

Head Difference (m):		2.0		Area of Sample (m ²)		4.191E-03	
Length of Sample (m):		7.960E-02		Gradient, i		2.562E+01	
Elapsed Time (Minutes)	Average Volume Change (mL)	Average Temperature (°C)		k _t (m/s)	R _T	k ₂₀ (m/s)	
1805	1.08	21.0		8.734E-11	0.976	8.525E-11	
2872	1.59	21.0		8.239E-11	0.976	8.041E-11	
3342	1.81	21.0		8.100E-11	0.976	7.906E-11	
4458	2.32	21.0		7.846E-11	0.976	7.658E-11	
6222	3.10	21.0		7.553E-11	0.976	7.372E-11	
7203	3.50	21.0		7.396E-11	0.976	7.218E-11	
8606	4.08	21.0		7.226E-11	0.976	7.053E-11	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	-		-	-	-	
-	-	AVERAGE		7.871E-11	-	7.682E-11	



Project Name:		Depth:	3.05 m
Project Number:	USG1276	Testing Company:	Union Street Geo.
Client:		Field Technician:	M.W.
Testhole:	BH109	Sample Date:	
Location:		Lab Technician:	B.B.
Sample Number:	MW48	Date Tested:	September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Material and Test Description

Material Description:			
Sandy Clay			
Test Type:	Constant Head	Remoulding Details	
Mould Size:	Flexible Wall	Max Dry Density (kg/m ³):	-
Sample Source:	Shelby Tube	Proctor ID:	-
Fluid Used:	Deaired Water	Percent Max (%):	-
Fluid Reservoir:	Burrettes	Target Dry Density (kg/m ³):	-

Initial Sample Characteristics

Water Content		Sample Size					
Wet + Tare (g):	607.6	Trial	1	2	3	4	Average
Dry + Tare (g):	490.7	Diameter (mm):	73.4	73	72.9	72.8	73.0
Tare (g):	14.0	Length (mm):	80.3	80.2	80.4	80	80.2
Water Content (%):	24.5%	Weight (g)	651.3				

Area (cm ²):	41.9	Specific Gravity (Note 2):	2.68
Volume (cm ³):	336.0	Void Ratio:	72.4%
Wet Density (kg/m ³):	1938	Saturation:	90.9%
Dry Density (kg/m ³):	1557	Porosity:	42.0%

Final Sample Characteristics

Water Content		Sample Size					
Wet + Tare (g):	864.4	Trial	1	2	3	4	Average
Dry + Tare (g):	732.4	Diameter (mm):	73.1	71.9	72.4	73.0	72.6
Tare (g):	199.6	Length (mm):	80.2	80.1	80	80.2	80.1
Water Content (%):	24.8%	Weight (g)	667.1				

Area (cm ²):	41.4	Specific Gravity (Note 1):	2.68
Volume (cm ³):	331.7	Void Ratio:	66.5%
Wet Density (kg/m ³):	2011	Saturation:	100.0%
Dry Density (kg/m ³):	1612	Porosity:	39.9%

Note 1: Specific gravity for final sample characteristics calculation adjusted to result in 100.0% saturation.

Note 2: Specific gravity for initial sample characteristics calculation set equal to that of the final.

Project Name: _____
 Project Number: USG1276
 Client: _____
 Testhole: BH109
 Location: _____
 Sample Number: MW48

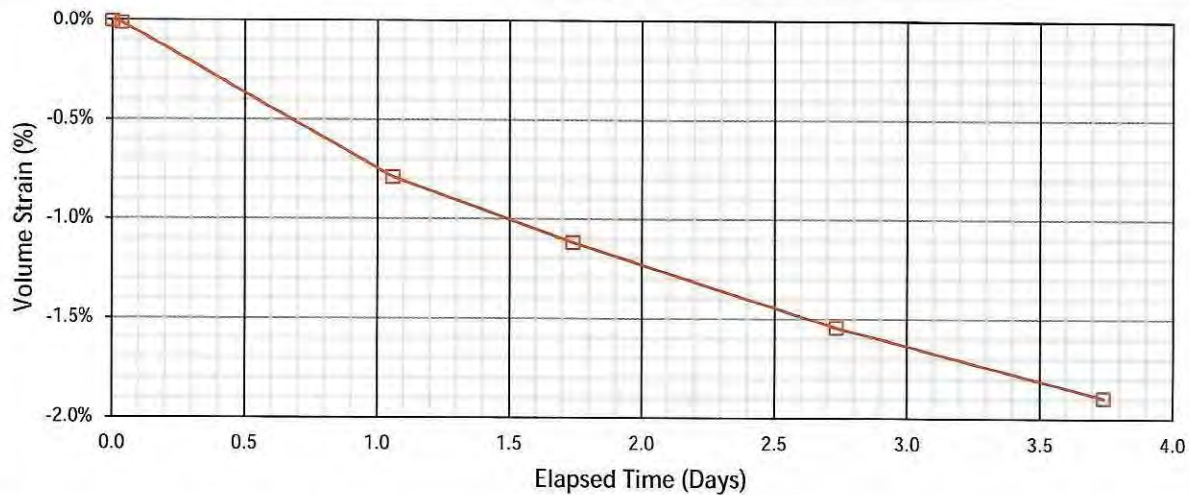
Depth: 3.05 m
 Testing Company: Union Street Geo.
 Field Technician: M.W.
 Sample Date: _____
 Lab Technician: B.B.
 Date Tested: September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Saturation Data

Cell Pressure (kPa):		160.0		Top Pressure (kPa):		130.0	
Bottom Pressure (kPa):		130.0		Pressure Difference (kPa):		-	
Date & Time	Elapsed Time (Days)	Room Temp (°C)	Top Burret (mL)	Bottom Burret (mL)	Cell (mL)	Total Vol. Change (mL)	Volume Strain (%)
9/18/21 14:08	0.00	21.0	3.4	3.4	11.9	0	0.00%
9/18/21 14:58	0.03	21.0	3.4	3.3	12.0	-0.04	-0.01%
9/19/21 15:33	1.06	21.0	3.3	3.3	14.8	-2.65	-0.79%
9/20/21 7:53	1.74	21.0	3.4	3.4	15.7	-3.76	-1.12%
9/21/21 7:46	2.73	21.0	3.5	3.6	16.8	-5.19	-1.54%
9/22/21 7:54	3.74	21.0	3.7	3.7	17.7	-6.36	-1.89%
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-



Project Name: _____
 Project Number: USG1276
 Client: _____
 Testhole: BH109
 Location: _____
 Sample Number: MW48

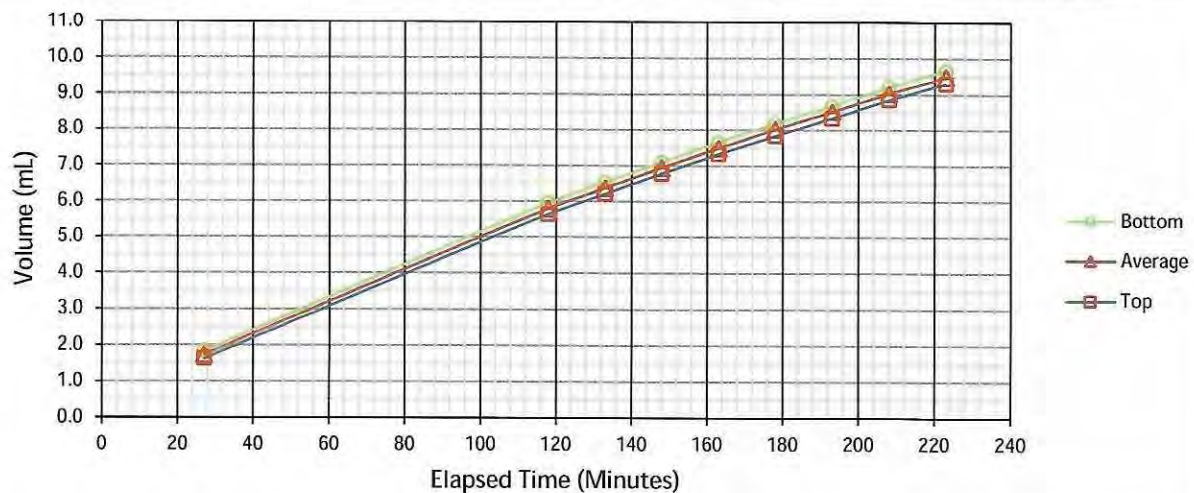
Depth: 3.05 m
 Testing Company: Union Street Geo.
 Field Technician: M.W.
 Sample Date: _____
 Lab Technician: B.B.
 Date Tested: September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Permeation Data

Cell Pressure (kPa):		160.0		Top Pressure (kPa):		120.0	
Bottom Pressure (kPa):		140.0		Pressure Difference (kPa):		20.0	
Date & Time	Elapsed Time (Minutes)	Room Temp (°C)	Top Buret (mL)	Bottom Buret (mL)	Top Vol. Change (mL)	Bottom Vol. Change (mL)	Average Vol. Change (mL)
9/22/21 7:59	0	21.0	9.77	0.10	0.00	0.00	0.00
9/22/21 8:26	27	21.0	8.14	1.94	1.63	1.84	1.74
9/22/21 9:57	118	21.0	4.13	6.06	5.64	5.96	5.80
9/22/21 10:12	133	21.0	3.55	6.64	6.22	6.54	6.38
9/22/21 10:27	148	21.0	2.99	7.20	6.78	7.10	6.94
9/22/21 10:42	163	21.0	2.45	7.75	7.32	7.65	7.49
9/22/21 10:57	178	21.0	1.94	8.28	7.83	8.18	8.01
9/22/21 11:12	193	21.0	1.44	8.78	8.33	8.68	8.51
9/22/21 11:27	208	21.0	0.92	9.31	8.85	9.21	9.03
9/22/21 11:42	223	21.0	0.47	9.77	9.30	9.67	9.49
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-



Project Name: _____
 Project Number: USG1276
 Client: _____
 Testhole: BH109
 Location: _____
 Sample Number: MW48

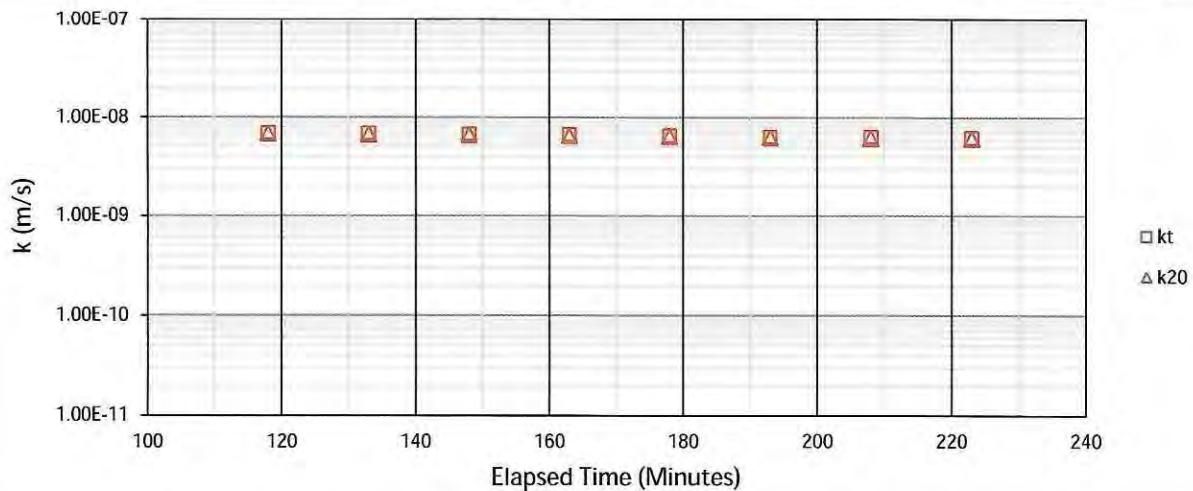
Depth: 3.05 m
 Testing Company: Union Street Geo.
 Field Technician: M.W.
 Sample Date: _____
 Lab Technician: B.B.
 Date Tested: September 18, 2021

Flexible Wall Permeameter (ASTM D5084-10)

Standard Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter

Permeation Data

Head Difference (m):		2.0		Area of Sample (m ²):		4.164E-03	
Length of Sample (m):		8.018E-02		Gradient, i:		2.544E+01	
Elapsed Time (Minutes)	Average Volume Change (mL)	Average Temperature (°C)	k _t (m/s)	R _T	k ₂₀ (m/s)		
118	5.80	21.0	7.028E-09	0.976	6.860E-09		
133	6.38	21.0	6.895E-09	0.976	6.729E-09		
148	6.94	21.0	6.768E-09	0.976	6.606E-09		
163	7.49	21.0	6.652E-09	0.976	6.493E-09		
178	8.01	21.0	6.533E-09	0.976	6.376E-09		
193	8.51	21.0	6.417E-09	0.976	6.263E-09		
208	9.03	21.0	6.341E-09	0.976	6.189E-09		
223	9.49	21.0	6.221E-09	0.976	6.072E-09		
-	-	-	-	-	-		
-	-	-	-	-	-		
-	-	-	-	-	-		
-	-	-	-	-	-		
-	-	-	-	-	-		
-	-	-	-	-	-		
-	-	AVERAGE	6.607E-09	-	6.448E-09		



Laboratory Hydrometer

Sample No.: MW07

Sample Information

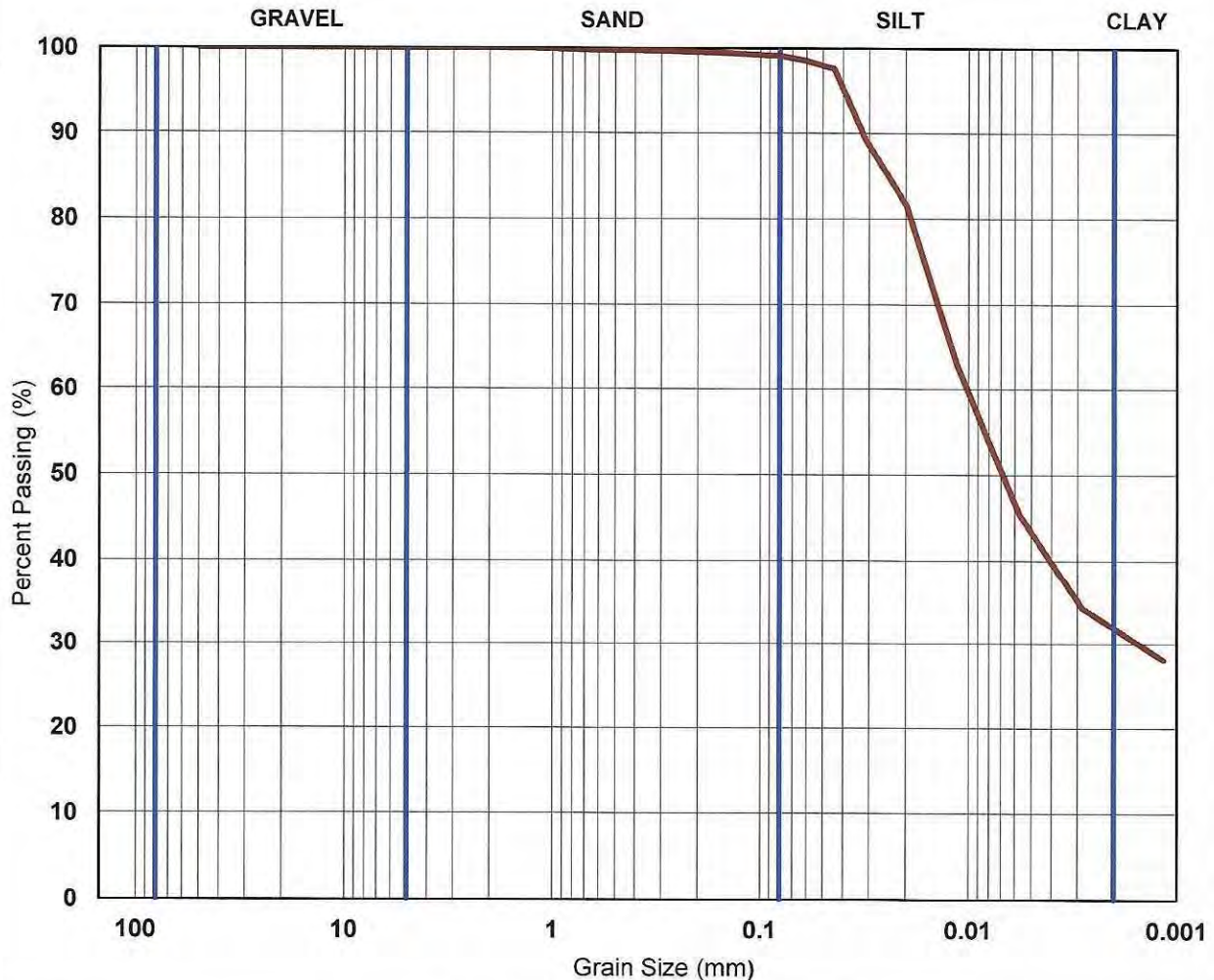
Date: 14 Sept., 2021 **By:** M.W. **of:** USG **Type:** Pail / Bag
Location: BH101, 17.5' **Specification:** ASTM D 422
Description: Silt, clayey, sand inclusions.

Specifications: Laboratory Specifications as per ASTM D 422.

Comments: _____

Sieve Results:

By Type (%): Gravel = 0.0 Sand = 0.9 Silt = 67.9 Clay = 31.2



CLIENT: Eagle Builders LP **FILE No.:** USG1276
PROJECT: 2021 Geotech Investigation **DATE:** 17-Sep-21
LOCATION: Red Deer, Alberta **TECH:** E.G.

March 7, 2022

Jeff Froese
NRCB Approval Officer
Red Deer, AB
jeff.froese@nrcb.ca

RE: Eagle Builders – Double T Cattle Feedlot – Application RA21043 – NE 15-42-25 W4

Thank you for your email request on this file regarding the storage volume of the proposed irregular shape catch basin.

As shown in Figure 1, the Al-Terra engineering report lists a HWL volume of 9650m³ at a liquid elevation of 864.0m.

Methodology

The chosen methodology to determine the storage volume is to “split” the irregular shape into two rectangular shapes, and calculate the volume for each shape (Figure 1):

1. Large, west rectangle. Dimensions would be:
 - a. Length of 128.38m (at 864.5m)
 - b. Width of 56.64m (at 864.5m)

2. Small, east rectangle. Dimensions would be:
 - a. Length of 52.21m (108.85 – 56.64), (at 864.5m)
 - b. Width of 21.99m (at 864.5m)

3. For both rectangles, common dimensions are:
 - a. Top of berm 864.5m
 - b. Liquid top 864.0m
 - c. Pond bottom is 862.5m
 - d. Freeboard depth is 0.5m
 - e. Total volume (including freeboard) is calculated using a depth of 2.0m
 - f. Usable volume (excluding freeboard) is calculated using a liquid depth of 1.5m
 - g. Interior side slopes are 3:1 (horizontal to vertical)

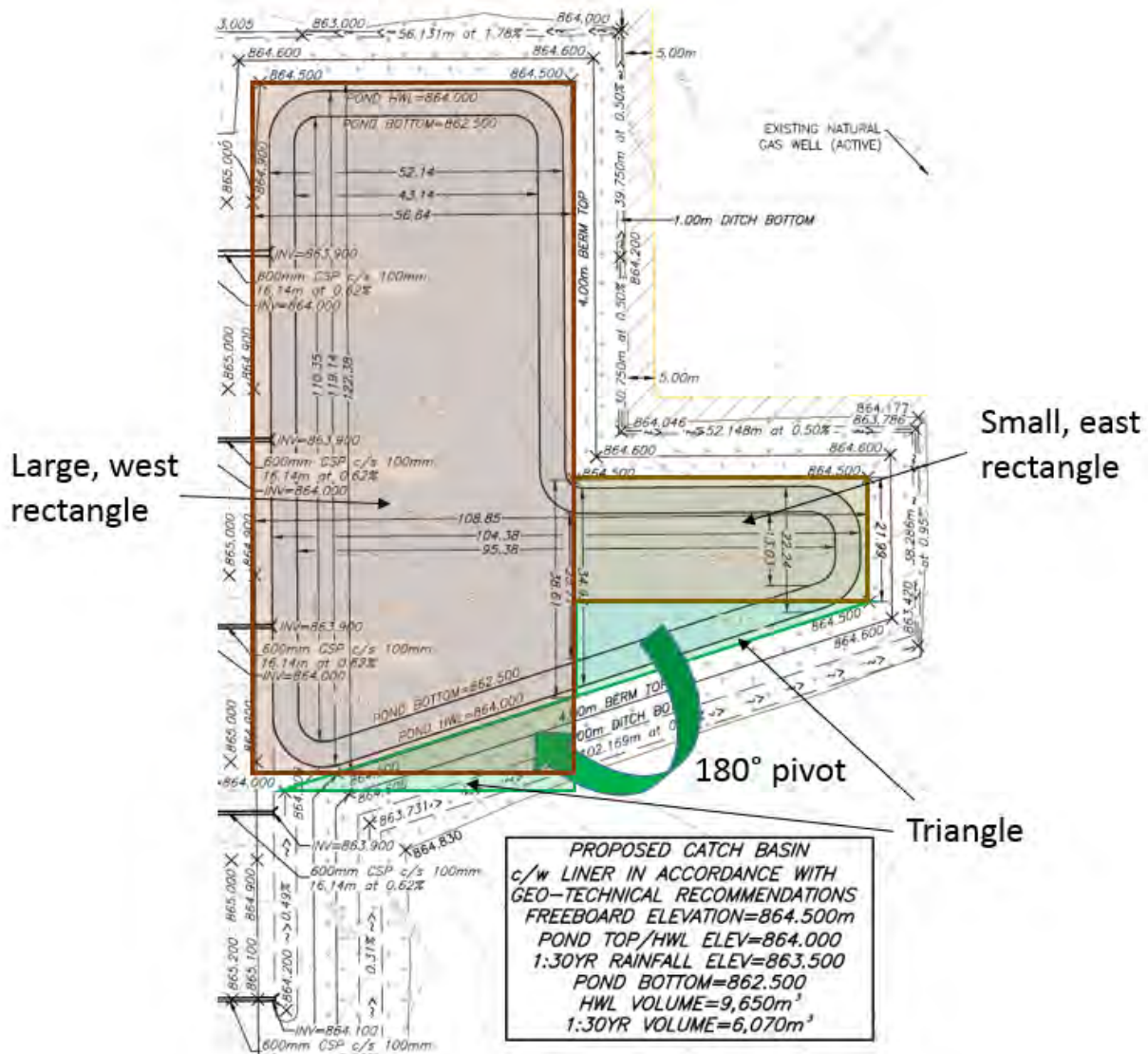


Figure 1. Irregular shape catch basin. Al-Terra Engineering (Red Deer) Ltd., Site Grading Plan Sheet C02. #5, 2022-02-23. Color shading and labels added.

Assumptions

Assumptions and their anticipated effects on the calculations:

- Rounded corners shown are calculated as “square”.
 - o This would result in a catch basin volume calculation being slightly larger than the actual catch basin volume.
- Angled catch basin slope on the southeast is split in approximately half, at the intersection of a line that would extend south from the interior berm east top of the large, west rectangle. The triangle on the east side of this slope (and south of the small, east rectangle) would pivot 180 degrees, and be added to the south east corner of the large, west rectangle. “Moving” this portion of the irregular shape catch basin would result in the dimensions of the two rectangles listed.

- As a portion of the triangle is shown beyond the dimensions of the large, west rectangle (Figure 1), this would result in a catch basin volume calculation being slightly smaller than the actual catch basin volume.
- Catch basin dimensions at west edge of entire catch basin are measured to gravel road (at 864.9m), rather than the catch basin top (at 864.5m) on the other sides.
 - This would result in a catch basin volume calculation slightly larger than the actual catch basin volume.
- Calculating the volume of two catch basins suggests that a “berm” (with 3:1 H to V side slopes) would be between them, when in fact the “berm” volume would be usable catch basin volume.
 - This would result in a catch basin volume calculation slightly smaller than the actual catch basin volume.

Calculations

Volume calculations of the rectangles were made using a calculator published by Alberta Agriculture, Food and Rural Economic Development (<https://www.agric.gov.ab.ca/app19/loadcatchbasin>)¹. Results for 1.5m of liquid storage (liquid elevation of 864.0m) for the large, west rectangle and small, east rectangle are shown in Appendix A, and are:

1. Large, west rectangle volume = 8885 m³ usable volume (excluding freeboard)
2. Small, east rectangle volume = 976 m³ usable volume (excluding freeboard)
3. Total = 9861 m³ usable volume (excluding freeboard)

Conclusion

Al-Terra provided a volume of 9650m³ at a liquid elevation of 864.0m. The total usable volume (excluding freeboard) using the two rectangles method was calculated to be 9861 m³. The slightly larger volume calculation associated with the two rectangles method is likely to be the net result of the four assumptions.

It is reasonable to use the Al-Terra volume of 9650m³ as the usable volume (excluding freeboard) of the irregular shape catch basin.

If you have any questions, please contact me.

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¹ Runoff potential was calculated separately because too many assumptions would have been necessary to determine the portion of the feedlot draining to the large, west and small, east rectangles. Therefore, runoff potential calculations were not included in Appendix A.

Catch Basin Dimension Calculator

For more information on runoff control catch basin design consideration including liner options, catch basin protection, etc., check out the catch basin [factsheet](#).

Name

Land Location

Estimating Runoff Potential

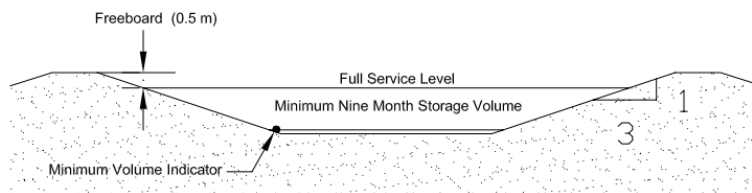
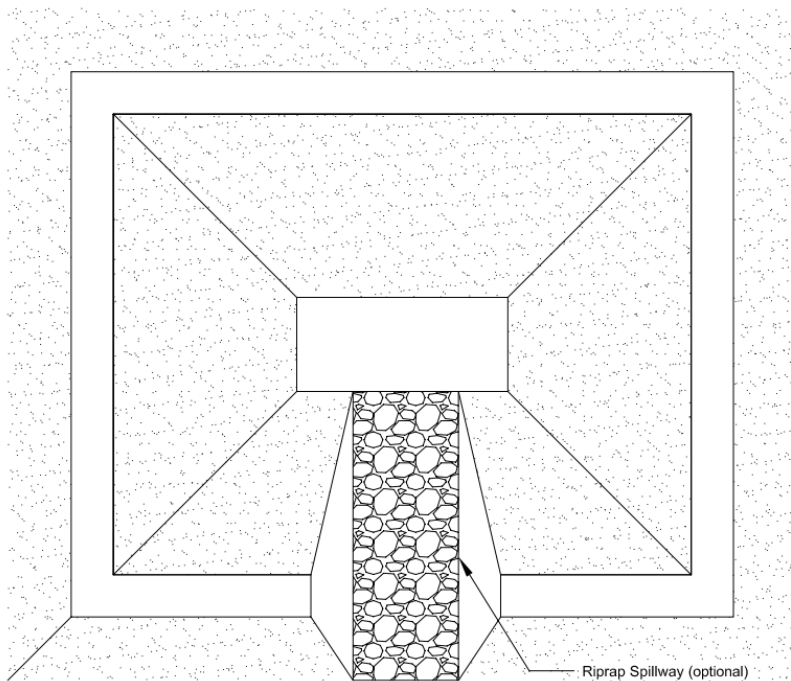
Area	Length (m)	Width (m)	Paved?	Area (m ²)
1	<input type="text"/>	<input type="text"/>	NO <input type="button" value="v"/>	
Total Area				

Calculating Catch Basin Volume:

Construction Dimensions	Storage Dimensions
Length (m): <input type="text" value="128"/>	125.0
Width (m): <input type="text" value="56.6"/>	53.6
Depth (m): <input type="text" value="2.0"/>	1.5

Evacuation Capacity:
 12370 m³
 436842 ft³
 2721015 Imp. Gal

Catch basin volume (minus freeboard):
 8885 m³
 313771 ft³
 1954427 Imp. Gal



Comparing Catch Basin Volume versus Runoff Potential:

Runoff potential: m^3
 Catch basin volume: 8885 m^3

The catch basin dimensions meet the design requirements in AOPA

Catch Basin Dimension Calculator

For more information on runoff control catch basin design consideration including liner options, catch basin protection, etc., check out the catch basin [factsheet](#).

Name

Land Location

Estimating Runoff Potential

Area	Length (m)	Width (m)	Paved?	Area (m ²)
1	<input type="text"/>	<input type="text"/>	NO <input type="button" value="v"/>	
Total Area				

Calculating Catch Basin Volume:

Construction Dimensions **Storage Dimensions**

Length (m): 49.2

Width (m): 18.9

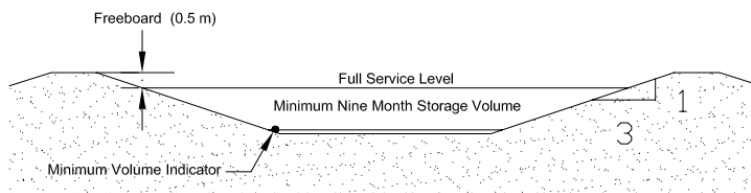
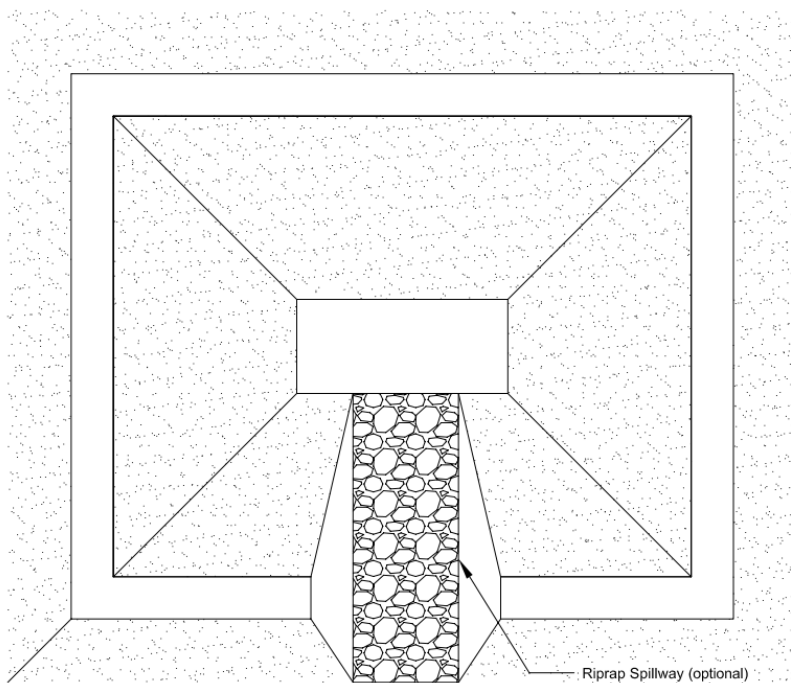
Depth (m): 1.5

Evacuation Capacity:

1493 m³
 52725 ft³
 328415 Imp. Gal

Catch basin volume (minus freeboard):

976 m³
 34467 ft³
 214689 Imp. Gal



Comparing Catch Basin Volume versus Runoff Potential:

Runoff potential: m³
 Catch basin volume: 976 m³

The catch basin dimensions meet the design requirements in AOPA

